

#### ADDENDUM NUMBER THREE

- Date: January 23, 2023
- From: McAdams Co. & Kimley-Horn and Associates
- Re: EB-5894 Black Creek Greenway BL-0024 – NW Cary Parkway Sidewalk at Black Creek

#### **NOTICE TO BIDDERS:**

Bidder is hereby notified that this Addendum shall become a part of the Contract Documents and shall be attached to the Project Manual for the Project.

The following items are intended to revise and clarify the Drawings and the Project Manual.

The bidder shall ensure that his Sub-Bidders are in full receipt of the information contained herein.

ADDENDA ITEMS:	
PROJECT MANUAL	(1) ITEMS
DRAWINGS	(0) ITEMS
MISCELLANEOUS	(1) ITEMS
RFI's	(25) ITEMS
END OF TITLE PAGE	



#### **PROJECT MANUAL**

1. Revised Precast Culvert Special Provision.

#### DRAWINGS

1. n/a

#### MISCELLANEOUS

1. Current Plan holders list

#### RFIs

**Q38.** For the bridge portion it has a undercut items and a select granular item. Is the select granular item the backfill for the undercut item or is backfill incidental?

Response: EB-5894 – Select granular material is for foundations and backfill of box culverts and pipes as required in the plans and specifications. Undercut shall use Class IV per section 500.

- Q40. The geotechnical report prepared by SM&E dated April 21, 2017, references a scour depth of 5-ft:
  - a. KNA is not estimating scour potential and will use that magnitude of scour at the top of the piles that are not protected by retaining walls. Please confirm this is acceptable.
     Response: EB-5894 – Assuming scour depth from the bottom of cap is acceptable.
  - b. How should we apply the scour depth to piles installed behind retaining walls? Scour in these cases affects the retaining walls, and if it is considered that the walls won't fail, piles shouldn't feel the effect of scour.

Response: EB-5894 – For piles protected by retaining walls, assume that scour is not applied to the piles.

- c. Could you please confirm if the approached used by the geotechnical engineer to model scour in LPILE was by using p multipliers equal to zero for the affected soil layers?
  Response: EB-5894 The Micropile Designer will make their own determination of scour effects on the pile analysis and submit the design for review as part of the submittal packages.
- Q41. Please confirm the type and quality of material, and construction sequence that will be used for MSE wall backfill at the bridge end bents.
  Response: EB-5894 Contractor to submit sealed shop drawings for any walls to also include proposed

backfill materials. Backfill for MSE walls outside the reinforced zone in accordance with Article 410-8 of the Standard Specifications

**Q42.** Micropiles being designed and to be installed by KNA are not planned to resist slope instabilities nor drag force due to the new fill. Please confirm this is acceptable.

Response: EB-5894 – Engineer agrees that retaining wall analysis and design should address slope stability. However, the down drag of new fill is a geotechnical consideration that should be included if significant. Please refer to the geotechnical report.

# **McAdams**

Q43. The geotechnical report prepared by SM&E dated April 21, 2017 mentions that the pile analysis performed assumed a fixed head condition for all structures. Looking at the pile connection details fixed head conditions are not met with the specified 1-ft embedment in end bent piles, and no embedment at the timber bents. Please confirm which condition we should assume for design and pricing purposes.

Response: EB-5894 – Pile heads were considered relatively "fixed" to the bent caps for lateral analysis.
 Q44. For several lateral load requirements, deflections shown in the abovementioned geotechnical report exceed the typical 0.5 inches acceptable for most structures (report shows up to 1 inch). If the models are run assuming a free-head conditions as KNA considers should be the case, deflections will be even greater. What is the lateral deflection limit for the different design loads provided?

Response: EB-5894 – The final design of the micropiles submitted by the Contractor for review shall meet the one-inch lateral deflection criteria in the Geotechnical report for the limit state force effects provided in the plans.

**Q45.** For the EB-5894 contract, only 1 verification test is required at Bridge A. Will any other testing be required at the other bridge or Boardwalk micropile locations?

Response: EB-5894 – Verification testing at one micropile supported bridge site (Bridge A) is required for this contract.

**Q47.** For Bridge B Bent 1; would an alternate casing with a thinner wall be accepted? 1" wall thickness is not standard for 80 ksi pipe.

Response: EB-5894 – The Special Provisions for the micropiles require the Contractor to provide a final design for review and acceptance. The PSPs allow for larger pile sizes (up to 3" greater nominal diameter) but not alternate pile types or layouts. Casing wall thicknesses may be adjusted with pile diameter to meet the design criteria in the PSPs, but not less than 0.5" thickness.

- Q48. Specifications call for Prime casing, would mill secondaries be allowed instead? Response: EB-5894 – Yes, the intent is for mill secondary steel casing to be allowed.
- Q49. The casing required for Bridge B Bent 1, would mill secondaries be permitted as Prime is not available? Response: EB-5894 – Same as above.
- **Q50.** For Bridge B Bent 1, would an alternative pile layout with additional plum or battered piles be permitted in lieu of nonstandard heavy wall pipe?

Response: EB-5894 – The Special Provisions for the Micropiles require the Contractor to provide a final design for review and acceptance. The PSPs allow for larger pile sizes (up to 3" greater nominal diameter) but not alternate pile types or layouts.

**Q51.** Micro piles- With mill secondary, and even prime oilfield type of casing materials (i.e. API grades), no guarantee can be given to us to how the galvanizing process will be finalized. Because of the chemistry of this type of casing, galvanizing sometimes will not adhere to the casing. Therefore, there is a possibility that additional charges could be applied in a change order. Would cold galvanizing (painting) be permitted from 1' below grade to the fully exposed pile be allowed?

### Response: EB-5894 – Follow the requirements of the contract documents for galvanizing.

- **Q52.** Please confirm the quantity and location of:
  - a. Demonstration piles
    Response: Demonstration micropiles and load tests are to be performed for bridge A at end bent
    2.
  - b. Verification load tests

Response: EB-5894 – To be proposed in the Contractor's testing plan in accordance with the PSPs.

c. Proof load tests Response: EB-5894 – To be proposed in the Contractor's testing plan in accordance with the PSPs.

# **McAdams**

- Q53. Do these micropiles have any design shear or uplift loads? If so, please provide the relevant information. Response: EB-5894 – The design factored force resistances are shown on the Foundation Layouts in the plans.
- Q62. The lateral design capacities required for the Bridge A and B micropiles (12 20 tons) are significant for an 8" diameter micropile. Can a larger diameter micropiles or drilled shafts be considered to combat the lateral design loads?

Response: EB-5894 – The Special Provisions for the Micropiles require the Contractor to include Micropile designer and provide a final design for review and acceptance. The PSPs allow for larger pile sizes (up to 3" greater nominal diameter), but not alternate pile types or layouts.

**Q63.** Is the micropile contractor responsible for designing the reinforcement casing size and thickness, center bar size, and bond length?

Response: EB-5894 – The Special Provisions require the Contractor to provide a Micropile designer, submit the final micropile design, installation and testing plan and other specified items for review. The minimum size of casing, center bar, and bond length for the micropiles are established in the plans and specifications. The final bar sizes and bond lengths are to be designed submitted for approval. The Contractor is responsible for providing an acceptable final design for the pile, casement, reinforcement, center bar and bond length of the piles to be provided. The final casing diameter if limited to 3" greater than in nominal size than the nominal size shown in the plans and not less than 0.5" thickness.

- Q64. The project documents indicate 1" thick reinforcement casing is required at Bridge B; it is likely that this wall thickness will not be readily available. Are other casing dimensions allowed so long as calculations indicate allowable stresses and deflections are satisfied?
  Response: EB-5894 Yes, not less than 0.5" thickness may be considered, along with increased diameter, as allowed above.
- Q65. A common micropile pipe OD is 7-5/8", please confirm this will be satisfactory for the stated 8" OD pipe. Response: EB-5894 – Common OD sizes of micropile casings of 7.625" and 8.625" (with at least 0.5" wall thickness per plans) are accepted as 8" nominal OD size. However, the Contractor must submit supporting engineering calculations and final design for the proposed micropiles to be included in the submittal package as referenced in Q63 above.
- Q66. Are threaded micropile joints acceptable? Response: EB-5894 – Yes, if the threading and coupling of the splice meet the API 5CT and ASME criteria for oil and gas casing pipe. Use of threaded couplings for splices will not relieve the Contractor from meeting any of the other requirements of the contract documents.
- **Q67.** Please confirm that the design lateral loads stated in the drawings are the total per location (end bent) and not a per pile design load.

Response: EB-5894 – Lateral Load resistance shown in the Foundation Layout notes is per pile.

- Q68. If the answer to our question #5 above is that the stated lateral loads are per pile, please provide the preliminary L-Pile analysis used for the geotechnical recommendations.
  Response: EB-5894 See appendices of Geotechnical Report.
- Q69. What is the allowable deflection at the top of the micropile for the stated lateral loads? Response: EB-5894 – one-inch of lateral deflection at the associated limit state resistance forces stated in the report.
- **Q70.** On page 341/634 of the project manual, it is stated that mill secondary pipe is acceptable. On page 403/634, it is stated that mill secondary pipe is not acceptable. Please clarify. Note that prime pipe is likely significantly more expensive than mill secondary.

Response: EB-5894 – Yes, the intent is for mill secondary steel casing to be allowed.



**Q71.** The project manual states "Use drilling methods that result in the annulus between reinforcing casings and the ground filled with grout." A drilling method to satisfy this statement is more costly and complicated compared to typical installation methods. Typically, reinforcement casing is duplex drilled in place, tight to the ground to minimum tip elevations or top of rock then the drill hole is advanced to the bottom of the bond zone. Please confirm that this alternate method is acceptable.

**Response: EB-5894 – Follow the requirements stated in the project manual.** 

**Q72.** Page 339 of 834 of the project manual, under 4.0 Basis of Payment, notes that Culvert Excavation and Foundation Conditioning Material associated with the 1 LS culverts will be paid for in accordance with those pay items. We do not see either of those pay items within the EB-5894 pay item list. Could those two items be added, or made incidental to the Culvert 1 LS items?

Response: EB-5894 – The Culvert Excavation and Foundation Conditioning Materials (if needed) shall be incidental to the precast culvert. The Special provision has been revised to reflect this change.

# PRECAST REINFORCED CONCRETE BOX CULVERT

# 1.0 General

This Special Provision covers the design, fabrication and construction of precast reinforced concrete box culverts intended for the conveyance of storm water.

When a precast reinforced concrete box culvert is required on the plans, design the precast culvert sections in accordance with ASTM C1577 or the current edition of the AASHTO LRFD Bridge Design Specifications. Rate all sizes of precast reinforced concrete box culverts in accordance with the current edition of the AASHTO Manual for Bridge Evaluation. Ensure the culvert rates for AASHTO H-10 truck as per The Town of Cary Standard Specifications (see 2020 Town of Cary Standard Specifications Section 09012 for Greenway Bridge design loads). Provide the size and number of barrels as indicated on the plans. Provide a precast box culvert that meets the requirements of the 2018 NCDOT Standard Specifications Section 1077 and any other applicable parts of the Standard Specifications.

The design and rating of the precast and cast-in-place members is the responsibility of the Contractor and is subject to review, comments and approval. Submit two sets of detailed plans and rating sheets for review. Include all details in the plans, including the size and spacing of the required reinforcement necessary to build the precast box and cast-in-place members. Have a North Carolina Registered Professional Engineer check and seal the plans, rating sheets and design calculations. After the plans, rating sheets and design calculations are reviewed and, if necessary, the corrections made, submit one set of plans and rating sheets on 22" x 34" sheets to become part of the contract plans.

If the span, rise and design earth cover for the precast reinforced concrete box culvert are identical to a previously approved submittal, the Contractor may request the previously approved design calculations and plans be considered as the submittal for review and approval. However, a set of plans and rating sheets will need to be submitted to become part of the contract plans.

### 2.0 Town of Cary's Greenway Legal Loads

Apply the following legal loads to all structures carrying greenway traffic:



# 3.0 Precast Reinforced Concrete Box Sections

The precast reinforced concrete box culvert sections shall match the size and hydraulic opening indicated in the contract plans.

### A. Design

- 1. Design Fill The design earth cover is reported on the plans as the elevation difference between the point of maximum fill and the bottom of the top slab.
- 2. Placement of Reinforcement Provide a 1 inch concrete cover over the reinforcement subject to the provisions of Section F. Extend the inside reinforcement into the tongue portion of the joint and the outside reinforcement into the groove portion of the joint. Detail the clear distance of the end wires so it is not less than 1/2 inch or more than 2 inches from the ends of the box section. Assemble reinforcement per the requirements of ASTM C1577 or the approved design. The exposure of the ends of the wires used to position the reinforcement is not a cause for rejection.
- 3. Laps and Spacing Use lap splices for the transverse reinforcement. Detail the transverse wires so that the center to center spacing is not less than 2 inches or more than 4 inches. Do not detail the longitudinal wires with a center to center spacing of more than 8 inches.

### B. Joints

1. Produce the precast reinforced concrete box section with tongue and groove ends. Design and form these ends of the box section so, when the sections are laid together, they make a

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continuous line of box sections with a smooth interior free of appreciable irregularities in the flowline, all compatible with the permissible variations given in Section F. The internal joint formed at the tongue and groove ends of the precast units shall be sealed with either bitumen/butyl sealant or closed-cell neoprene material. The internal joint material shall be installed in accordance with the manufacturer's recommendations. The material shall be shown on the shop drawings when they are submitted for review.

2. Seal the external joint with an outside sealer wrap conforming to ASTM C877 that is at least 12 inches wide and covers the joint on both the sides and the top of the box section. Use ConWrap CS-212 from Concrete Sealants, Inc., EZ-Wrap from Press-Seal Gasket Corporation, Seal Wrap from Mar-Mac Manufacturing Co., Inc., Cadilloc External Pipe Joint from Cadilloc, or an approved equal for the outside sealer wrap. If the outside sealer wrap is not applied in a continuous strip along the entire joint, a 12 inch minimum lap of the outside sealer wrap is permitted. Before placing the outside sealer wrap, clean and prime the area receiving the outside sealer wrap in accordance with the sealer wrap manufacturer recommendations. The joint wrap manufacturer installation recommendations shall be included with shop drawings submitted for review. The external joint wrap shall be installed in pieces, as indicated on Figure 1 below:



# Figure 1

Cover the external joint sealer with a 3 foot strip of filter fabric conforming to Type 4 requirements in Section 1056 of the Standard Specifications.

Place multiple lines of a precast reinforced concrete box culvert such that the longitudinal joint between the sections has a minimum width of 3 inches. Fill the joint between multiple lines of precast box sections with Class A concrete. Use Class A concrete that meets the requirements listed in the Standard Specifications except that Field Compressive Strength Specimens are not required.

# C. Manufacture

Manufacture precast reinforced concrete box culvert sections by either the wet cast method or dry cast method.

1. Mixture - In addition to the requirements of Section 1077 of the Standard Specifications, do

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not proportion the mix with less than 564 lb/yd<sup>3</sup> of portland cement.

- Strength Concrete shall develop a minimum 28-day compressive strength of 5000 psi. Movement of the precast sections should be minimized during the initial curing period. Any damage caused by moving or handling during the initial curing phase will be grounds for rejection of that precast section.
- 3. Air Entrainment Air entrain the concrete in accordance with Section 1077 5(A) of the 2012 NCDOT Standard Specifications. For dry cast manufacturing, air entrainment is not required.
- 4. Testing Test the concrete in accordance with the requirements of the Standard Specifications Section 1077 5(B).
- 5. Handling Handling devices or holes are permitted in each box section for the purpose of handling and placing. Submit details of handling devices or holes for approval and do not cast any concrete until approval is granted. Remove all handling devices flush with concrete surfaces as directed. Fill holes in a neat and workmanlike manner with an approved non-metallic non-shrink grout, concrete, or hole plug.
- D. Physical Requirements

Acceptability of precast culvert sections is based on concrete cylinders made and tested in accordance with ASTM C31 and ASTM C39.

- E. Permissible Variations
  - 1. Flatness All external surfaces shall be flat, true, and plumb. Irregularities, depressions, or high spots on all external surfaces shall not exceed 1/2 inch in 8 feet.
  - 2. Internal Dimensions Produce sections so that the internal and haunch dimensions do not vary more than 1/4 inch from the plan dimensions.
  - 3. Adjacent Sections Internal, external, and haunch dimensions for connecting sections shall not vary more than 1/2 inch.
  - 4. Length of Tongue and Groove The minimum length of the tongue shall be 4 inches. The minimum length of the groove shall be 4 inches. The dimensions of the tongue and groove shall not vary more than 1/4 inch from the plan dimensions.
  - 5. Slab and Wall Thickness Produce sections so that the slab and wall thickness are not less than that shown on the plans by more than 5% or 3/16 inch, whichever is greater. A thickness more than that required on the plans is not a cause for rejection.
  - 6. Length of Opposite Surfaces Produce sections so that variations in laying lengths of two opposite surfaces of the box section meet the requirements of ASTM C1577, Section 11.3.
  - 7. Length of Section Produce sections so that the underrun in length of a section is not more than 1/2 inch in any box section.
  - 8. Position of Reinforcement Produce sections so that the maximum variation in the position

of the reinforcement is  $\pm 3/8$  inch for slab and wall thicknesses of 5 inches or less and  $\pm 1/2$  inch for slab and wall thicknesses greater than 5 inches. Produce sections so that the concrete cover is never less than 5/8 inch as measured to the internal surface or the external surface. The preceding minimum cover limitations do not apply at the mating surfaces of the joint.

9. Area of Reinforcement – Use the design steel shown on the plans for the steel reinforcement. Steel areas greater than those required are not cause for rejection. The permissible variation in diameter of any wire in finished fabric is prescribed for the wire before fabrication by either AASHTO M32 or M225.

# F. Marking

- 1. Each section shall be match-marked in order of intended installation as indicated on the approved shop drawings. Ensure that pieces fit together neatly and in a workmanlike manner. In order to ensure a good, neat field fit, the Department will verify assembly of the first five adjacent sections or 20% of the total culvert length, whichever is greater, at the producer's facility and match-mark the pieces. This will require that a minimum of three adjacent sections of the culvert be fitted at the production yard at a time and then match-marked. Once three sections have been match-marked, the first section may be removed for shipment and a fourth section set for marking. Continue in a progressive manner until all sections have been properly match-marked. The producer shall document the GO-NO-GO dimensional measurements of each box culvert section produced through the post-pour inspection process.
- 2. Clearly mark each section of the box culvert in accordance with ASTM C1577, Section 15. The information requirements of Section 15.1 shall be clearly marked on the inner surface of each section.

# G. Construction

- 1. Pre-installation Meeting A pre-installation meeting is required prior to installation. Representatives from the Contractor, the precast box manufacturer, and the Town should attend this meeting. The precast box manufacturer representative shall be on site during installation.
- 2. Foundation Foundation for precast box culvert shall meet the requirements of Section 414 of the Standard Specifications. In addition, Type VI foundation material shall be encapsulated in filter fabric conforming to Type 4 requirements in Section 1056 of the Standard Specifications. The filter fabric shall be placed perpendicular to the culvert barrel. Provide sufficient overhang beyond the excavation to allow a minimum lap of 3 feet when the foundation material is placed and fabric wrapped on top. Perpendicular sections of fabric shall be continuous. A minimum lap of 2 feet shall be provided between sections of fabric.
- 3. Installation Sections shall be placed at the beginning of the outlet end of the culvert with the groove end being laid upgrade. Tongue sections shall be laid into the groove sections. Positive means shall be provided to pull each section firmly into the previously placed section so that the joints are tightly homed. Use a "come-along", box pullers or other approved methods to create a positive means of joining box sections. Construction equipment shall not have direct contact with the box section. The load of the box shall be suspended by lifting device during joining procedure.

4. Backfill – Complete backfill in accordance with Section 414 of the Standard Specifications.

# 4.0 Basis of Payment

The Precast Reinforced Concrete Box Culvert as described on the plans and in this Special Provision will be paid for at the contract lump sum price for "Precast Reinforced Concrete Box Culvert at Station 15+31.14". Such price and payment will be full compensation for all work covered by this Special Provision, the plans and applicable parts of the Standard Specifications and will include, but not be limited to, furnishing all labor, materials (including all filter fabric), equipment and other incidentals necessary to complete this work. Such price and payment will also be full compensation for concrete, reinforcing steel, labor, equipment and all other related materials necessary for the completion of the barrel section, and the construction of the headwalls, end curtain walls, wings and wing footings. Culvert Excavation and Foundation Conditioning Material, if needed, shall be executed in accordance with the Standard Specifications but will be incidental to the precast culvert.

Payment will be made under:

Precast Reinforced Concrete Box Culvert Double 4'x8' at Station L1 15+31	.Lump Sum
Precast Reinforced Concrete Box Culvert 3'x5' at Station L1 41+40	.Lump Sum
Precast Reinforced Concrete Box Culvert 4'x12' at Station L1 70+20	.Lump Sum

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