

UNIVERSITY OF NORTH CAROLINA AT WILMINGTON
PURCHASING SERVICES

IMPORTANT ADDENDUM

Date Addendum Issued: 9/18/23

FAILURE TO RETURN THIS SIGNED ADDENDUM IN ACCORDANCE WITH INSTRUCTIONS MAY SUBJECT YOUR BID/PROPOSAL TO REJECTION.

Bid/RFP No. 72-BRAB24010

Addendum No.: 02

Purchasing Agent: Robert Bisanar

Commodity: AV integration services for new
Shoreline Sports Complex
Pavilion

Using Agency: UNCW

Opening Date: Remotely 9/21/23 via Zoom meeting
at @ 2:00 PM ET

I. INSTRUCTIONS:

This Addendum forms a part of the Bidding Documents and modifies the Contract Documents with amendments and additions noted below. See attached portico structural steel (6-page) layout for mounting hard points for speakers and/or lights.

Execute Addendum:

VENDOR: _____

AUTHORIZED SIGNATURE: _____ DATE: _____

NAME and TITLE (Typed): _____

ADDRESS (CITY, STATE & ZIP): _____

THE LINE SHOWN ABOVE IS EXACTLY ONE INCH LONG AT THIS SHEET'S ORIGINAL PAPER SIZE

D

C

B

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17/Jun/22 10:03:22 PM

GENERAL NOTES:

1. THE STRUCTURAL DRAWINGS MUST BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS, AND THE SPECIFICATIONS. THE CONTRACTOR MUST VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, HOLES, AND ADDITIONAL ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.
2. THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE PROVISIONS OF THE NORTH CAROLINA STATE BUILDING CODE, 2018 EDITION.
3. THE CONTRACTOR MUST PROVIDE TEMPORARY SHORING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL PERMANENT SUPPORTS AND LATERAL BRACING ARE IN PLACE.
4. THE CONTRACTOR MUST FIELD VERIFY THE DIMENSIONS, ELEVATIONS, AND OTHER REQUIREMENTS NECESSARY FOR THE PROPER CONSTRUCTION AND ALIGNMENT OF THE STRUCTURE.
5. DISCREPANCIES BETWEEN DRAWINGS, BETWEEN THE DRAWINGS AND THE SPECIFICATIONS, OR WITHIN THE SPECIFICATIONS, MUST BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER DURING THE BIDDING PROCESS IN TIME TO PERMIT CLARIFICATION BY ADDENDUM. IF INCONSISTENCIES, DISCREPANCIES OR CONTRADICTIONS IN THE CONTRACT DOCUMENTS ARE DISCOVERED AFTER THE CLOSE OF BIDDING QUESTIONS, THE CONTRACTOR SHALL BE DEEMED BY SUBMITTAL OF HIS BID, TO HAVE BID THE MOST COSTLY AS TO LABOR, MATERIALS, DURATION, SEQUENCE AND METHOD OF CONSTRUCTION TO PROVIDE THE WORK.

6. DESIGN CRITERIA:

CLASSIFICATION OF BUILDING
RISK CATEGORY..... II

LIVE LOADS - UNIFORM:
SLAB ON GRADE..... 100 PSF
ROOF..... 20 PSF

SNOW LOADS:
GROUND SNOW LOAD..... 10 PSF
FLAT ROOF LOAD..... 7 PSF
IMPORTANCE FACTOR (Is)..... 1.0
THERMAL FACTOR (Ct)..... 1.0
EXPOSURE FACTOR (Ce)..... 1.0

WIND LOADS:
ULTIMATE DESIGN WIND SPEED (VULT)..... 146 MPH
EXPOSURE CATEGORY..... C
INTERNAL PRESSURE COEFFICIENT..... [±0.18]
COMPONENT AND CLADDING PRESSURES MAIN BUILDING:
WALLS, ZONE 5 (10 SF)..... XX PSF
ROOF, ZONE 3 (10 SF)..... XX PSF
PARAPET, END/CORNER (10 SF)..... XX PSF
ULTIMATE WIND BASE SHEARS (FOR MWFRS) MAIN BUILDING:
[Vx][VE-W]..... XX KIPS
[Vy][VN-S]..... XX KIPS
COMPONENT AND CLADDING PRESSURES PAVILION:
WALLS, ZONE 5 (10 SF)..... XX PSF
ROOF, ZONE 3 (10 SF)..... XX PSF
PARAPET, END/CORNER (10 SF)..... XX PSF
ULTIMATE WIND BASE SHEARS (FOR MWFRS) PAVILION:
[Vx][VE-W]..... XX KIPS
[Vy][VN-S]..... XX KIPS

SEISMIC LOADS:
SITE CLASSIFICATION..... D
SEISMIC DESIGN CATEGORY..... X
IMPORTANCE FACTOR (IE)..... 1.0
SPECTRAL RESPONSE ACCELERATIONS:
Ss..... X S1..... X
Sms..... X SM1..... X
Sps..... X SD1..... X
ANALYSIS PROCEDURE..... EQUIVALENT LATERAL FORCE
LATERAL FORCE RESISTING SYSTEM..... LIGHT-FRAMED WALLS
SHEATHED WITH WOOD
STRUCTURAL PANELS
RATED FOR SHEAR
RESPONSE MODIFICATION COEFFICIENT (R)..... 6.5
SEISMIC RESPONSE COEFFICIENT (Cs)..... XXX
ULTIMATE SEISMIC BASE SHEAR (V)..... XX KIPS

LATERAL DESIGN CONTROL
CONTROLLING LATERAL LOADS..... [WIND] [SEISMIC]

FOUNDATION NOTES:

1. FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING REPORT PREPARED BY ECU SOUTHEAST, LLP PROJECT # 22:30265, DATED MAY 6, 2021.
2. FOUNDATIONS HAVE BEEN DESIGNED FOR A NET ALLOWABLE SOIL BEARING PRESSURE OF [XX] PSF.
3. PRIOR TO PLACING FOUNDATION CONCRETE, ALL FOUNDATION EXCAVATIONS MUST BE INSPECTED BY THE OWNER'S GEOTECHNICAL TESTING AGENCY TO EXPLORE THE EXTENT OF LOOSE, SOFT, EXPANSIVE, OR OTHERWISE UNSATISFACTORY SOIL MATERIAL AND TO VERIFY DESIGN BEARING PRESSURE. DIRECTION FOR CORRECTIVE ACTION WILL BE PROVIDED BY THE SPECIAL INSPECTOR WHERE UNSATISFACTORY SOILS ARE PRESENT.
4. NO UNBALANCED BACKFILLING MUST BE DONE AGAINST MASONRY OR CONCRETE WALLS UNLESS WALLS ARE SECURELY BRACED AGAINST OVERTURNING, EITHER BY TEMPORARY CONSTRUCTION BRACING OR BY PERMANENT CONSTRUCTION.
5. CONTROL GROUNDWATER AND SURFACE RUNOFF THROUGHOUT THE CONSTRUCTION PROCESS. INUNDATION AND LONG TERM EXPOSURE OF BEARING SURFACES WHICH RESULT IN DETERIORATION OF BEARING MUST BE PREVENTED.

STRUCTURAL STEEL NOTES:

1. STRUCTURAL STEEL MUST BE IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 360.
2. STRUCTURAL STEEL MUST COMPLY WITH THE FOLLOWING SPECIFICATIONS:
A. STRUCTURAL STEEL SHAPES, PLATES AND BARS UNLESS OTHERWISE NOTED - ASTM A 36, Fy = 36 KSI
B. HOLLOW STRUCTURAL SECTIONS (HSS):
a. SQUARE AND RECTANGULAR - ASTM A 500, GRADE C, Fy = 50 KSI
3. WELDING MUST BE IN ACCORDANCE WITH AWS D1.1, "STRUCTURAL WELDING CODE - STEEL." WELD ELECTRODES MUST BE E70XX LOW HYDROGEN, UNLESS OTHERWISE NOTED, PROVIDE CONTINUOUS FILLET WELDS WITH MINIMUM SIZE REQUIRED BY TABLE J2.4 AISC 360.
4. HOT-DIP GALVANIZE AFTER FABRICATION THE FOLLOWING:
A. ANGLES AND PLATES SUPPORTING MASONRY IN EXTERIOR WALLS.
B. LINTELS AND LINTEL ASSEMBLIES SUPPORTING MASONRY IN EXTERIOR WALLS.

CAST-IN-PLACE CONCRETE NOTES:

1. CONCRETE MUST BE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301 AND 318.
2. CONCRETE MUST BE NORMAL WEIGHT AND MUST OBTAIN A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 4,000 PSI.
3. REINFORCING MATERIALS MUST BE AS FOLLOWS:
A. REINFORCING BARS - ASTM A615, GRADE 60, DEFORMED.
B. WELDED WIRE REINFORCEMENT - ASTM A1064, WELDED STEEL WIRE REINFORCEMENT; PROVIDE SHEET TYPE, ROLL TYPE IS NOT ACCEPTABLE.
4. ALL REINFORCING STEEL AND EMBEDDED ITEMS SUCH AS ANCHOR RODS AND WELD PLATES MUST BE ACCURATELY PLACED AND ADEQUATELY TIED AND SUPPORTED BEFORE CONCRETE IS PLACED TO PREVENT DISPLACEMENT BEYOND PERMITTED TOLERANCES.
5. CONCRETE COVER TO REINFORCING STEEL MUST CONFORM TO THE MINIMUM COVER RECOMMENDATIONS IN ACI 318, UNLESS THE DRAWINGS SHOW GREATER COVER REQUIREMENTS.
6. LAP CONTINUOUS REINFORCING STEEL 57 X BAR DIAMETER, TYPICAL UNLESS OTHERWISE NOTED.
7. CONCRETE IN AREAS EXPOSED TO WEATHER (SUCH AS EXTERIOR SLABS-ON-GRADE) MUST BE AIR ENTRAINED.

ROUGH CARPENTRY NOTES:

1. ROUGH CARPENTRY MUST BE IN ACCORDANCE WITH THE AMERICAN WOOD COUNCIL (AWC) "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION."
2. UNLESS OTHERWISE NOTED, USE 'COMMON' NAILS AND ALL NAILING MUST CONFORM TO THE "FASTENING SCHEDULE" TABLE 2304.10.1 OF THE BUILDING CODE.
3. WOOD FRAMING MEMBERS MUST COMPLY WITH PS 20 "AMERICAN SOFTWOOD LUMBER STANDARD" AND THE FOLLOWING REQUIREMENTS:
A. MOISTURE CONTENT - SEASONED, WITH 15 PERCENT MAXIMUM MOISTURE CONTENT.
B. GRADE - NO. 2, OR BETTER, UNLESS OTHERWISE NOTED.
C. SPECIES - SOUTHERN PINE UNDER WWPA RULES.
4. WOOD STRUCTURAL PANELS (WSP) MUST COMPLY WITH PS 1 "U.S. PRODUCT STANDARD FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD" FOR PLYWOOD CONSTRUCTION PANELS AND THE FOLLOWING REQUIREMENTS:
A. EXTERIOR WALL AND SHEAR WALL SHEATHING: 15/32 INCH, APA RATED SHEATHING, EXPOSURE 1 DURABILITY CLASSIFICATION.
B. ROOF SHEATHING: 5/8" INCH, APA RATED SHEATHING, EXPOSURE 1 DURABILITY CLASSIFICATION. PROVIDE TONGUE-AND-GROOVE EDGES OR USE "PLY-CLIPS" AT MID-SPAN BETWEEN EACH SUPPORT.
5. ALL WOOD FRAMING MEMBERS PERMANENTLY EXPOSED TO THE WEATHER AND ALL BEAMS, POSTS, AND SILL PLATES IN CONTACT WITH CONCRETE MUST BE PRESERVATIVE-TREATED. REFER TO THE SPECIFICATIONS.
6. STEEL PLATE CONNECTORS MUST COMPLY WITH ASTM A 36 SPECIFICATIONS (Fy= 36 KSI). BOLTS CONNECTING WOOD MEMBERS MUST COMPLY WITH ASTM A307 COMMON STEEL BOLTS, AND MUST BE 1/2 INCH DIAMETER, UNLESS OTHERWISE NOTED.
7. METAL FRAMING ANCHORS, HOLD DOWNS, HURRICANE TIES, HANGERS, ETC. MUST COMPLY WITH ASTM A653 AND BE CAPABLE OF SUPPORTING THE REACTIONS SHOWN. WHERE PRODUCTS OF A SPECIFIC MANUFACTURER ARE SHOWN, EQUAL PRODUCTS OF ANOTHER MANUFACTURER MAY BE USED. SUBJECT TO ENGINEER APPROVAL.
8. PROVIDE BRIDGING FOR ALL ROOF RAFTERS. MAXIMUM SPACING MUST BE 8'-0", UNLESS OTHERWISE NOTED.
9. PROVIDE HEADERS OF THE SAME CROSS SECTION AS JOISTS OR RAFTERS TO FRAME AROUND ALL OPENINGS TO SUPPORT SHEATHING, UNLESS OTHERWISE NOTED OR DETAILED ON THE DRAWINGS.
10. WHERE MULTIPLE FRAMING MEMBERS ARE INDICATED, SCAB CONTINGENT MEMBERS TOGETHER WITH 16d NAILS AT 12 INCHES ON CENTER, ALTERNATING AT 2 INCHES FROM EACH EDGE.
11. ALL CONNECTION HARDWARE IN CONTACT WITH PRESERVATIVE TREATED WOOD MUST BE HOT-DIP GALVANIZED COATED.

PREFABRICATED METAL-PLATE-CONNECTED WOOD TRUSS NOTES:

1. PREFABRICATED METAL-PLATE-CONNECTED WOOD TRUSSES MUST BE IN ACCORDANCE WITH THE AMERICAN WOOD COUNCIL (AWC) "NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION" AND THE TRUSS PLATE INSTITUTE (TPI) "NATIONAL DESIGN STANDARDS FOR METAL-PLATE-CONNECTED WOOD TRUSS CONSTRUCTION".
2. SUBMIT SHOP DRAWINGS SIGNED AND SEALED BY A NORTH CAROLINA LICENSED PROFESSIONAL ENGINEER RESPONSIBLE FOR THE DESIGN OF PREFABRICATED METAL-PLATE-CONNECTED WOOD TRUSSES. SHOP DRAWINGS MUST INCLUDE DESIGN LOADINGS AND REACTIONS APPLIED TO THE SUPPORTING STRUCTURE. INCLUDE ALL TRUSS SPLICE DETAILS AND TRUSS-TO-TRUSS CONNECTION DETAILS. SECONDARY BENDING STRESSES IN TRUSS TOP AND BOTTOM CHORDS DUE TO MEMBER LOADS MUST BE CONSIDERED IN THE DESIGN.
3. WOOD TRUSS FRAMING MEMBERS MUST COMPLY WITH PS 20 "AMERICAN SOFTWOOD LUMBER STANDARD" AND MUST BE SIZED BY THE MANUFACTURER FOR THE LOADS INDICATED.
4. METAL CONNECTOR PLATES MUST COMPLY WITH ASTM A653, GRADE A WITH COATING AS SPECIFIED.
5. WOOD TRUSS DESIGN LOADS MUST BE AS INDICATED IN "GENERAL NOTES" AND AS FOLLOWS:
A. TOP CHORD DEAD LOAD: 10 PSF (PLUS ADDITIONAL 5 PSF AT SUPERIMPOSED ROOF FRAMING AREAS).
B. WIND LOAD: WHEN CALCULATING NET UPLIFT REACTIONS, USE MAXIMUM RESISTING DEAD LOAD = 7 PSF ON TOP CHORD AND 0 PSF ON BOTTOM CHORD.
C. BOTTOM CHORD DEAD LOAD: 10 PSF.
6. WHERE MULTIPLE TRUSSES ARE INDICATED, SCAB CONTINGENT TRUSS MEMBERS TOGETHER WITH 16d NAILS AT 12 INCHES ON CENTER.
7. IN ADDITION TO THE TRUSS BRACING SHOWN, THE CONTRACTOR MUST PROVIDE ALL TEMPORARY AND PERMANENT BRACING AS REQUIRED FOR SAFE ERECTION OF THE TRUSSES, OR AS RECOMMENDED BY THE MANUFACTURER. THE GUIDELINES SET FORTH IN THE TRUSS PLATE INSTITUTE PUBLICATION "BRACING WOOD TRUSSES, COMMENTARY AND RECOMMENDATIONS" MUST BE CONSIDERED AS MINIMUM REQUIREMENTS.
8. PROVIDE ALL ADDITIONAL BRACING OF BOTH WEB AND CHORD MEMBERS REQUIRED BY THE TRUSS SHOP DRAWINGS.
9. ALL CONNECTION HARDWARE FOR TRUSS-TO-TRUSS CONNECTIONS AND TRUSS TO SUPPORTING STRUCTURE CONNECTIONS MUST BE SUPPLIED BY THE MANUFACTURER.

McADAMS

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601 S. COLLEGE ROAD
WILMINGTON, NC 28403
PHONE: 910.962.3000

UNCW Walton Drive
Student Village Recreation
Facilities Phase 4
1156 Walton Drive Wilmington , NC 28403
LS3P PROJECT: 7202-217800

REVISIONS:		
Δ	DATE	DESCRIPTION

PLAN INFORMATION	
SCO NO.	21-23669-018
PROJECT NO.	UNW21001
FILENAME	
CHECKED BY	Checker
DRAWN BY	
SCALE	12" = 1'-0"
DATE	06/17/2022

SHEET
GENERAL NOTES

S001

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THE LINE SHOWN ABOVE IS EXACTLY ONE INCH LONG AT THIS SHEET'S ORIGINAL PAPER SIZE

POST-INSTALLED ANCHOR NOTES:

1.

ALL POST INSTALLED ANCHORS INDICATED ON THE DRAWINGS ARE BY HILTI, INC, AND MUST BE CONSIDERED THE BASIS OF DESIGN PRODUCT. WHERE NOT EXPLICITLY INDICATED IN THE DRAWINGS, THE FOLLOWING ANCHORS/ADHESIVES MUST BE USED:

A.

ANCHORAGE TO CONCRETE

1.

ADHESIVE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE:

a.

HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) AND VC 20/40 VACUUM SYSTEM (VC 20-U OR VC 40-U) WITH STEEL THREADED ROD PER ICC ESR-3187.

2.

SCREW ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE:

a.

HILTI KWIK HUS EZ SCREW ANCHORS PER ICC ESR-3027.

B.

REBAR DOWELING INTO CONCRETE

1.

ADHESIVE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE:

a.

HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) AND VC 20/40 VACUUM SYSTEM (VC 20-U OR VC 40-U) WITH CONTINUOUSLY DEFORMED REBAR PER ICC ESR-3187.

C.

ANCHORAGE TO SOLID GROUTED MASONRY

1.

ADHESIVE ANCHORS USE:

a.

HILTI HIT-HY 270 MASONRY ADHESIVE ANCHORING SYSTEM (ICC PENDING).

b.

STEEL ANCHOR ELEMENT MUST BE HILTI HAS-E CONTINUOUSLY THREADED ROD.

2.

MECHANICAL ANCHORS USE:

a.

HILTI KWIK HUS EZ SCREW ANCHORS PER ICC ESR 3056.

D.

ANCHORAGE TO HOLLOW / MULTI-WYTHE MASONRY

1.

ADHESIVE ANCHORS USE:

a.

HILTI HIT-HY 270 MASONRY ADHESIVE ANCHORING SYSTEM PER ICC ESR-3342.

b.

STEEL ANCHOR ELEMENT MUST BE HILTI HAS-E CONTINUOUSLY THREADED ROD OR CONTINUOUSLY DEFORMED STEEL REBAR.

c.

THE APPROPRIATE SIZE SCREEN TUBE MUST BE USED PER ADHESIVE MANUFACTURER'S RECOMMENDATION.

2.

ALTERNATE POST INSTALLED ANCHOR PRODUCTS MAY BE SUBMITTED TO THE ENGINEER FOR REVIEW AND POSSIBLE APPROVAL. ALL SUBSTITUTION REQUESTS MUST BE ACCOMPANIED BY AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE. ALTERNATE PRODUCTS MAY REQUIRE MODIFICATIONS TO ANCHOR DIAMETER, SPACING, AND EMBEDMENT.

3.

INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING.

4.

THE CONTRACTOR MUST ARRANGE FOR AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF ANCHOR INSTALLATION.

5.

ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.

6.

EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR MUST LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY FERROSCAN OR GPR.

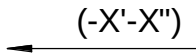
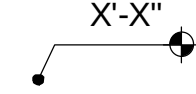
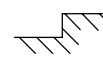
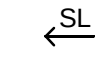
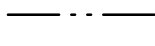
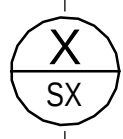
7.

ALL POST INSTALLED ANCHORS REQUIRE CONTINUOUS INSPECTIONS BY THE OWNER'S MATERIALS TESTING AGENCY TO VERIFY INSTALLATION HAS BEEN PERFORMED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS.

ABBREVIATIONS:

AFF	ABOVE FINISHED FLOOR	HVY	HEAVY
ARCH	ARCHITECT	INT	INTERIOR
BD	BAR DIAMETER	JBE	JOIST BEARING ELEVATION
BF	BRACED FRAME	JT	JOINT
BEJ	BUILDING EXPANSION JOINT	KCJ	KEYED CONSTRUCTION JOINT
BLDG	BUILDING	L	LOW
BM	BEAM	LLH	LONG LEG HORIZONTAL
BOD	BOTTOM OF DECK	LLV	LONG LEG VERTICAL
BOT, B	BOTTOM	LSH	LONG SIDE HORIZONTAL
BRG	BEARING	LSV	LONG SIDE VERTICAL
BTWN	BETWEEN	LTWT	LIGHTWEIGHT
C TO C	CENTER TO CENTER	LWC	LIGHTWEIGHT CONCRETE
CFMF	COLD-FORMED METAL FRAMING	MAS	MASONRY
		MATL	MATERIAL
CJ	CONTROL JOINT	MAX	MAXIMUM
CL	CENTERLINE	MECH	MECHANICAL
CLR	CLEAR	MF	MOMENT FRAME
CMU	CONCRETE MASONRY UNIT	MFR	MANUFACTURER
COL	COLUMN	MID	MIDDLE
CONC	CONCRETE	MIN	MINIMUM
CONN	CONNECTION	MOD	MODIFY
CONSTR	CONSTRUCTION	MOS	MIDDEPTH OF SLAB
CONT	CONTINUOUS	NOM	NOMINAL
COORD	COORDINATE	NTS	NOT TO SCALE
CTR	CENTER	OC	ON CENTER
CTRD	CENTERED	OPH	OPPOSITE HAND
CW	CURTAIN WALL	OPNG	OPENING
DBL	DOUBLE	PAF	POWDER ACTUATED FASTENER
DCJ	DOWELED CONSTRUCTION JOINT	PAR	PARALLEL
DIA, Ø	DIAMETER	PC	PIECE
DJ	DOUBLE JOIST	PEMB	PRE-ENGINEERED METAL BUILDING
DWGS	DRAWINGS	PEN	PENETRATE, PENETRATION
EA	EACH	PERP	PERPENDICULAR
EF	EACH FACE	PL	PLATE
EJ	EXPANSION JOINT	R	RADIUS
EL	ELEVATION	REF	REFERENCE, REFER TO
ELEV	ELEVATOR	REINF	REINFORCE, REINFORCED, REINFORCING
EMBED	EMBEDMENT		REQUIRED
EOD	EDGE OF DECK	REQD	REQUIREMENTS
EOS	EDGE OF SLAB	REQMTS	REQUIREMENTS
EQ	EQUAL	SCHED	SCHEDULE
EW	EACH WAY	SF	STEPPED FOOTING
EXIST	EXISTING	SGB	STEPPED GRADE BEAM
EXP	EXPANSION	SIM	SIMILAR
EXT	EXTERIOR	SJ	SAWED JOINT
FD	FLOOR DRAIN	SL	SLOPE
FDN	FOUNDATION	SOG	SLAB-ON-GRADE
FO	FACE OF	SPF	SIDEPLATE FRAME
FF EL	FINISHED FLOOR ELEVATION	STD	STANDARD
		TBE	TRUSS BEARING ELEVATION
FIN	FINISH	T&B	TOP & BOTTOM
FIN FLR	FINISHED FLOOR	T&G	TONGUE AND GROOVE
FOB	FACE OF BUILDING	THK	THICKNESS
FOC	FACE OF CONCRETE	TOC	TOP OF CONCRETE
FOM	FACE OF MASONRY	TOF	TOP OF FOOTING
FOS	FACE OF SLAB/ STUD	TOM	TOP OF MASONRY
FRMG	FRAMING	TOP	TOP OF PEDESTAL
FTG	FOOTING	TOS	TOP OF STEEL
FV, ±	FIELD VERIFY	TS	THICKENED SLAB
GALV	GALVANIZED	TYP	TYPICAL
GEN	GENERAL	UON	UNLESS OTHERWISE NOTED
GR BM	GRADE BEAM	VERT	VERTICAL
H	HIGH	W/	WITH
HK	HOOK	WP	WORKING POINT
HORIZ	HORIZONTAL	WSP	WOOD STRUCTURAL PANEL(S)
HSS	HOLLOW STRUCTURAL SECTION	WWR	WELDED WIRE REINFORCING
HT	HEIGHT		

PLAN LEGEND:

TBE = +X'-X"	=	TRUSS BEARING ELEVATION MEASURED FROM REFERENCED FINISHED FIRST FLOOR ELEVATION = 0'-0"
	=	TOP OF FOOTING ELEVATION MEASURED FROM REFERENCED FINISHED FIRST FLOOR ELEVATION = 0'-0"
	=	TOP OF SLAB ELEVATION MEASURED FROM REFERENCED FINISHED FIRST FLOOR ELEVATION = 0'-0"
	=	CHANGE IN ELEVATION - REF ARCH DWGS FOR DIMENSIONS
	=	DIRECTION OF SLOPE
	=	KCJ, OR SJ LINE ON PLAN
	=	PLAN KEY NOTE MARK
	=	COLUMN GRID MARK
	=	SECTION/DETAIL NUMBER/LETTER
	=	SECTION/DETAIL MARK
	=	SHEET NUMBER WHERE SECTION/DETAIL MARK IS DRAWN
H-X	=	WOOD HEADER MARK
WFX	=	WALL FOOTING MARK
CFX	=	COLUMN FOOTING MARK
	=	ROOF HIP / RIDGE LINE
	=	HOLD DOWN ANCHOR



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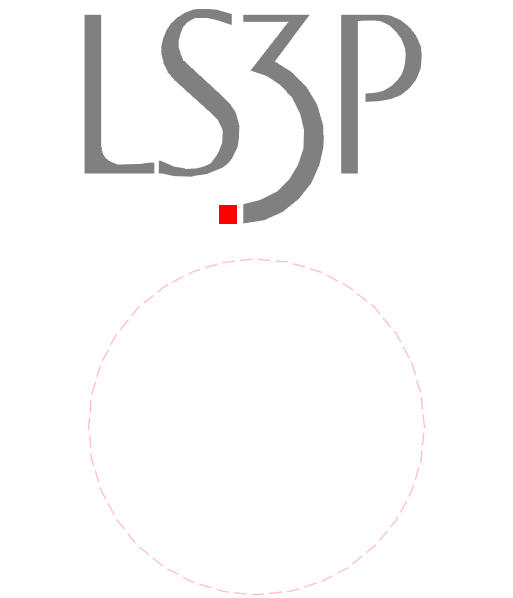
CLIENT

UNIVERSITY OF NORTH
CAROLINA WILMINGTON
601 S. COLLEGE ROAD
WILMINGTON, NC 28403
PHONE: 910.962.3000



UNCW Walton Drive
Student Village Recreation
Facilities Phase 4

1156 Walton Drive Wilmington , NC 28403
LS3P PROJECT: 7202-217800



REVISIONS:		
Δ	DATE	DESCRIPTION

PLAN INFORMATION		
SCO NO.	21-23669-01B	
PROJECT NO.	UNW21001	
FILENAME		
CHECKED BY	Checker	
DRAWN BY	Author	
SCALE	12" = 1'-0"	
DATE	06/17/2022	

SHEET

GENERAL NOTES

S002



COLUMN FOOTING SCHEDULE						
MARK	SIZE			REINFORCING		REMARKS
	LENGTH	WIDTH	DEPTH	BOTTOM	TOP	
CF5	5' - 0"	5' - 0"	1'-6"	(6)-#5 EA WAY	(6)-#5 EA WAY	-
CF6	6' - 0"	6' - 0"	1'-6"	(7)-#5 EA WAY	(7)-#5 EA WAY	-

THE LINE SHOWN ABOVE IS EXACTLY
ONE FOOT LONG AT THIS SHEET'S
ORIGINAL PAPER SIZE

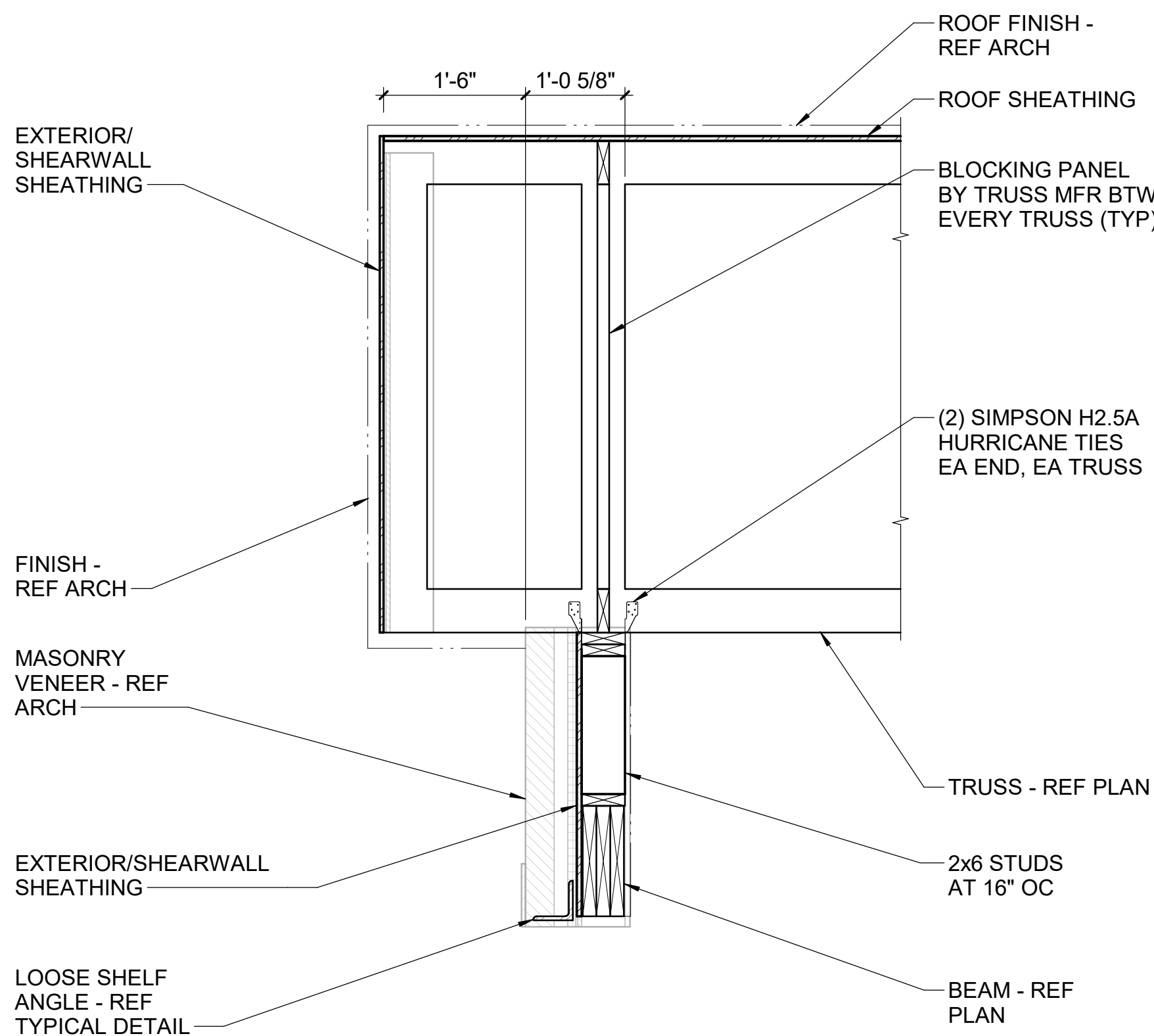
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C

B

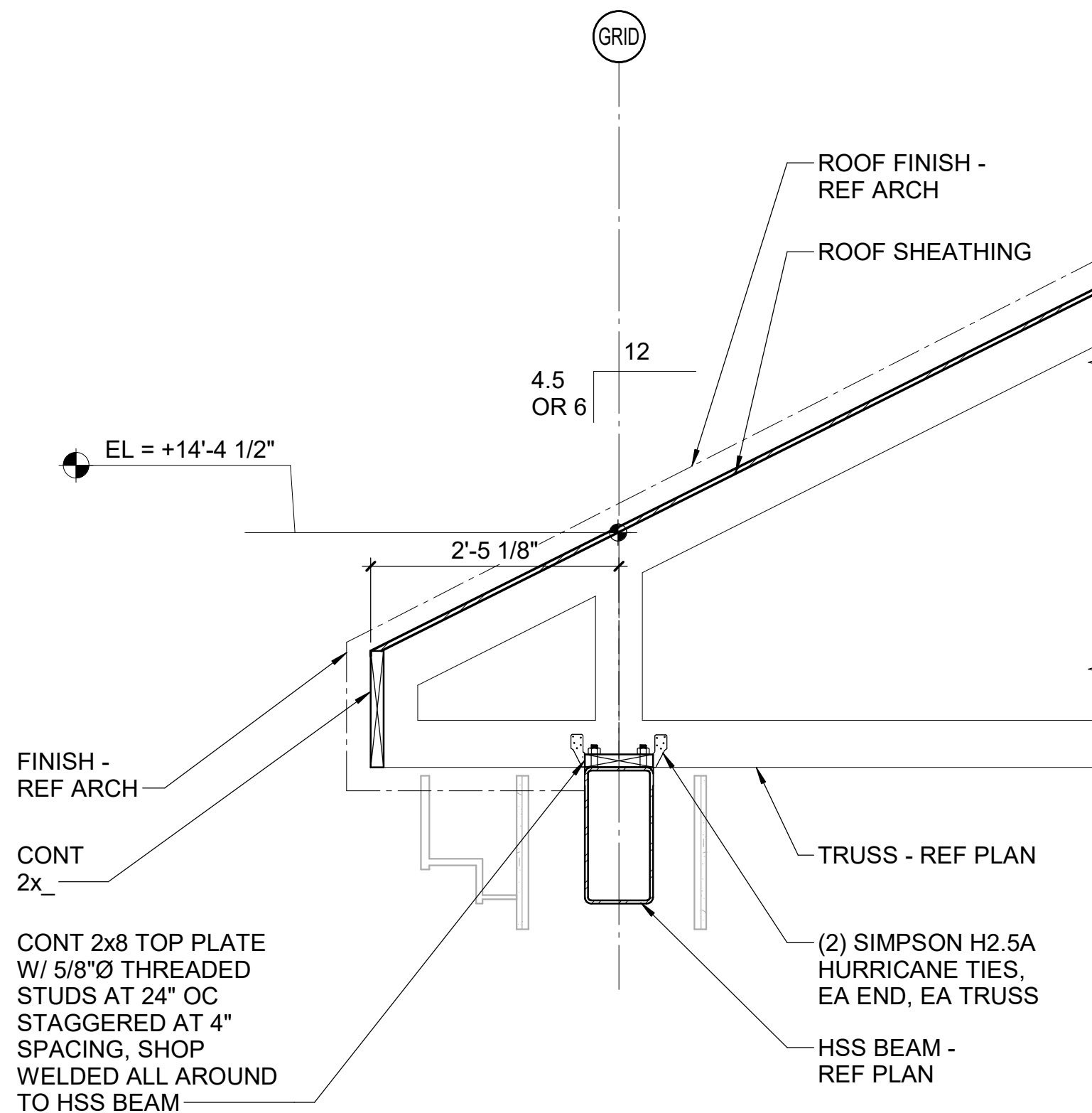
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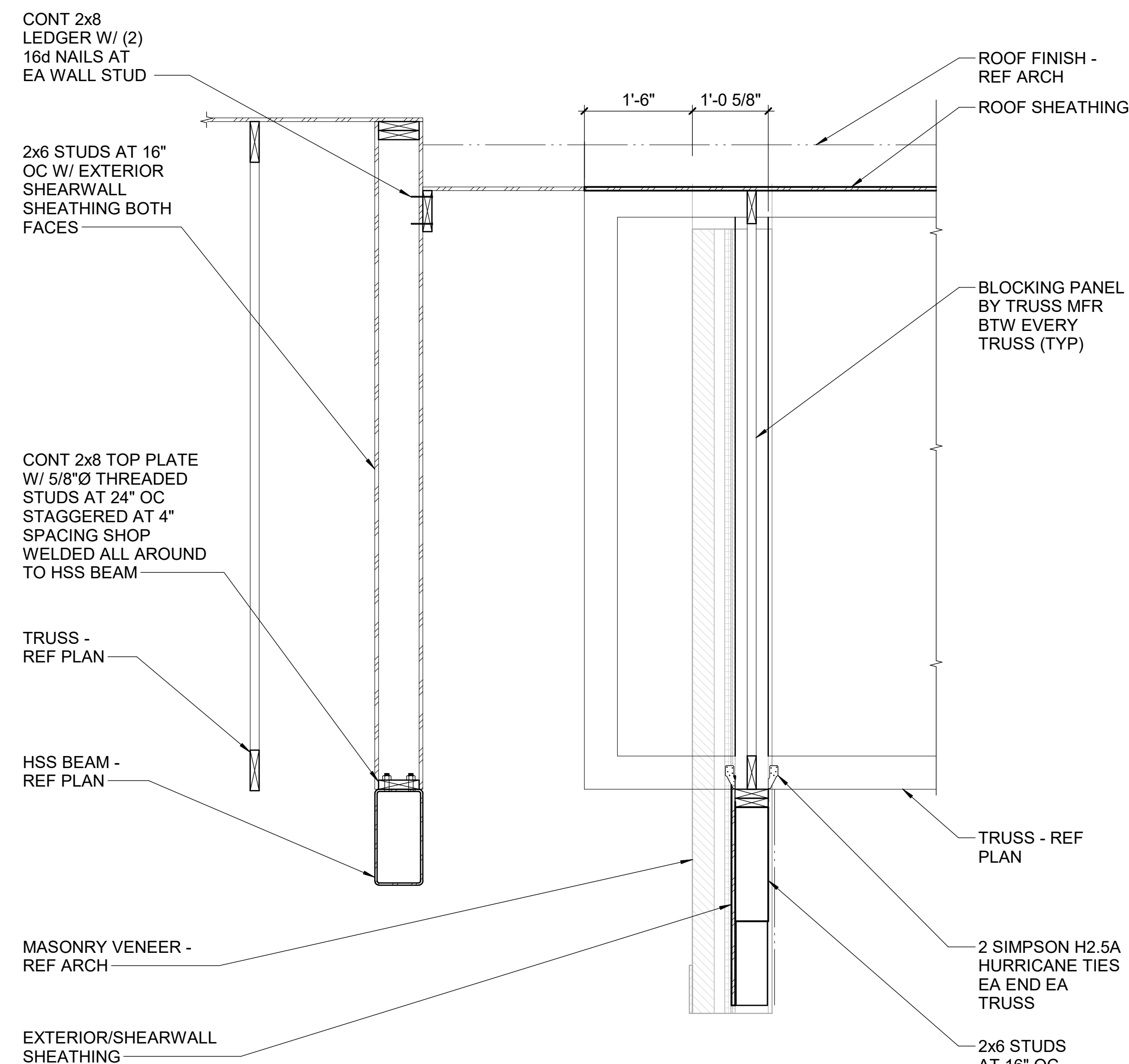
4 SECTION

3/4" = 1'-0"



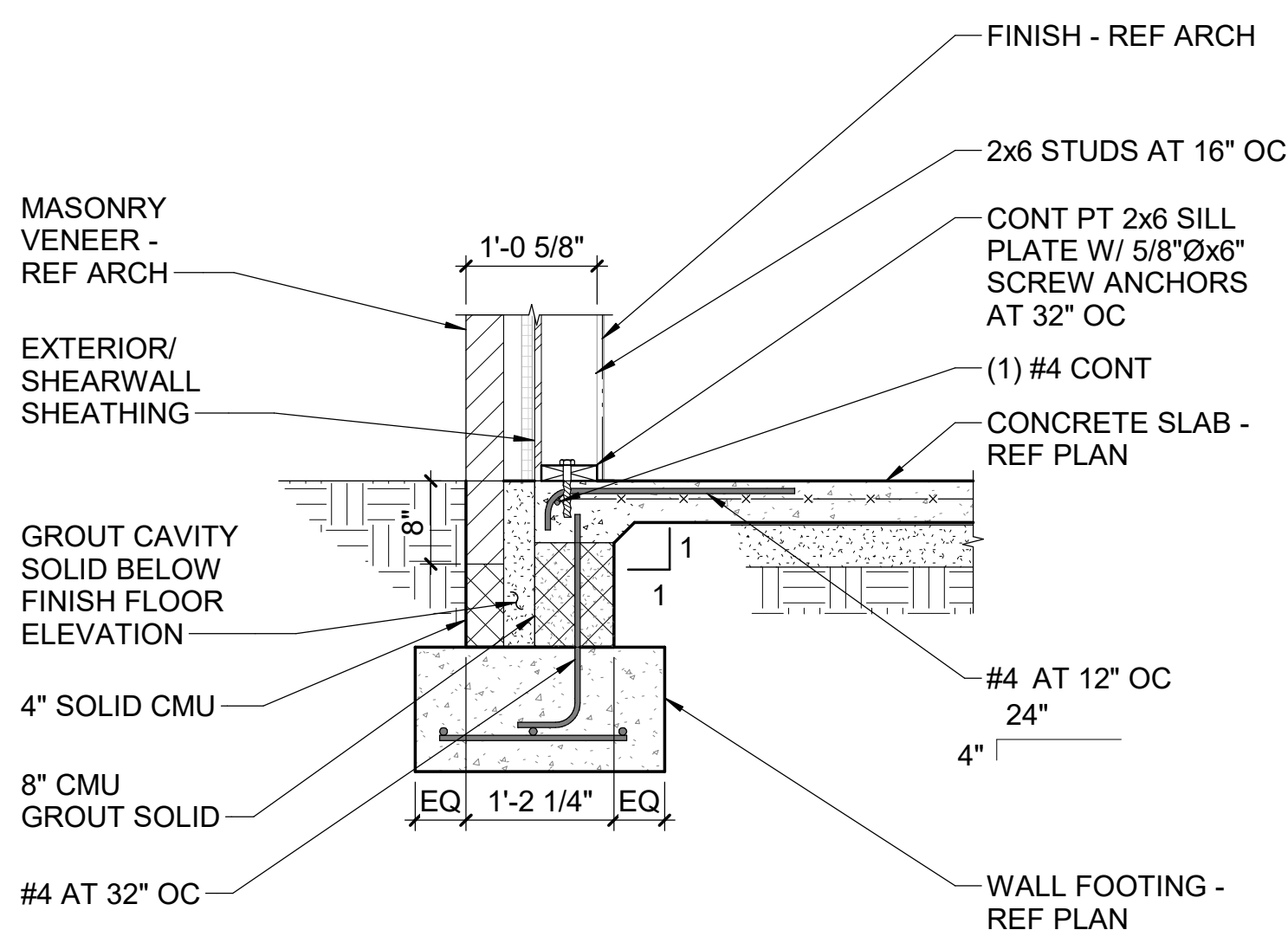
5 SECTION

3/4" = 1'-0"



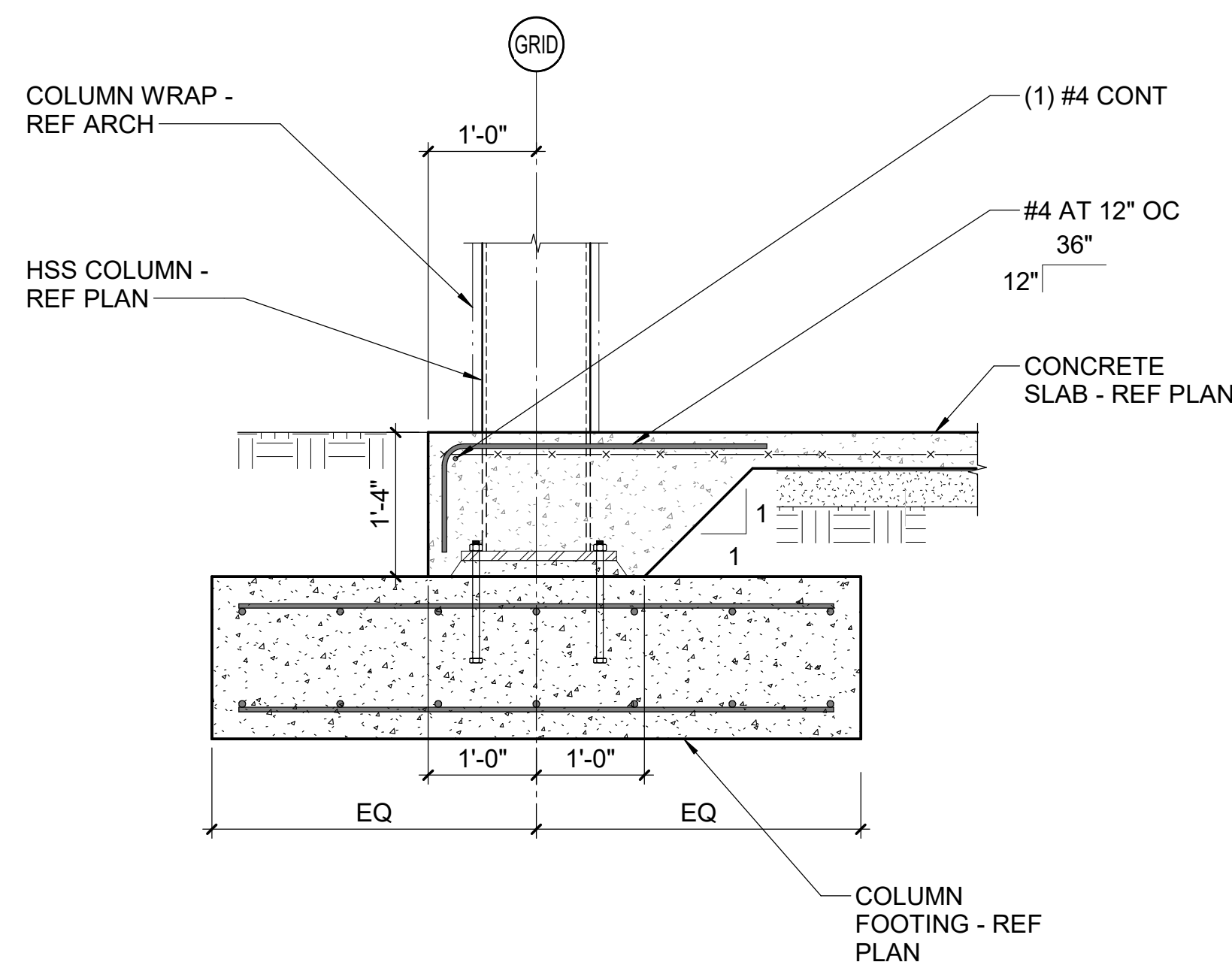
6 SECTION

3/4" = 1'-0"



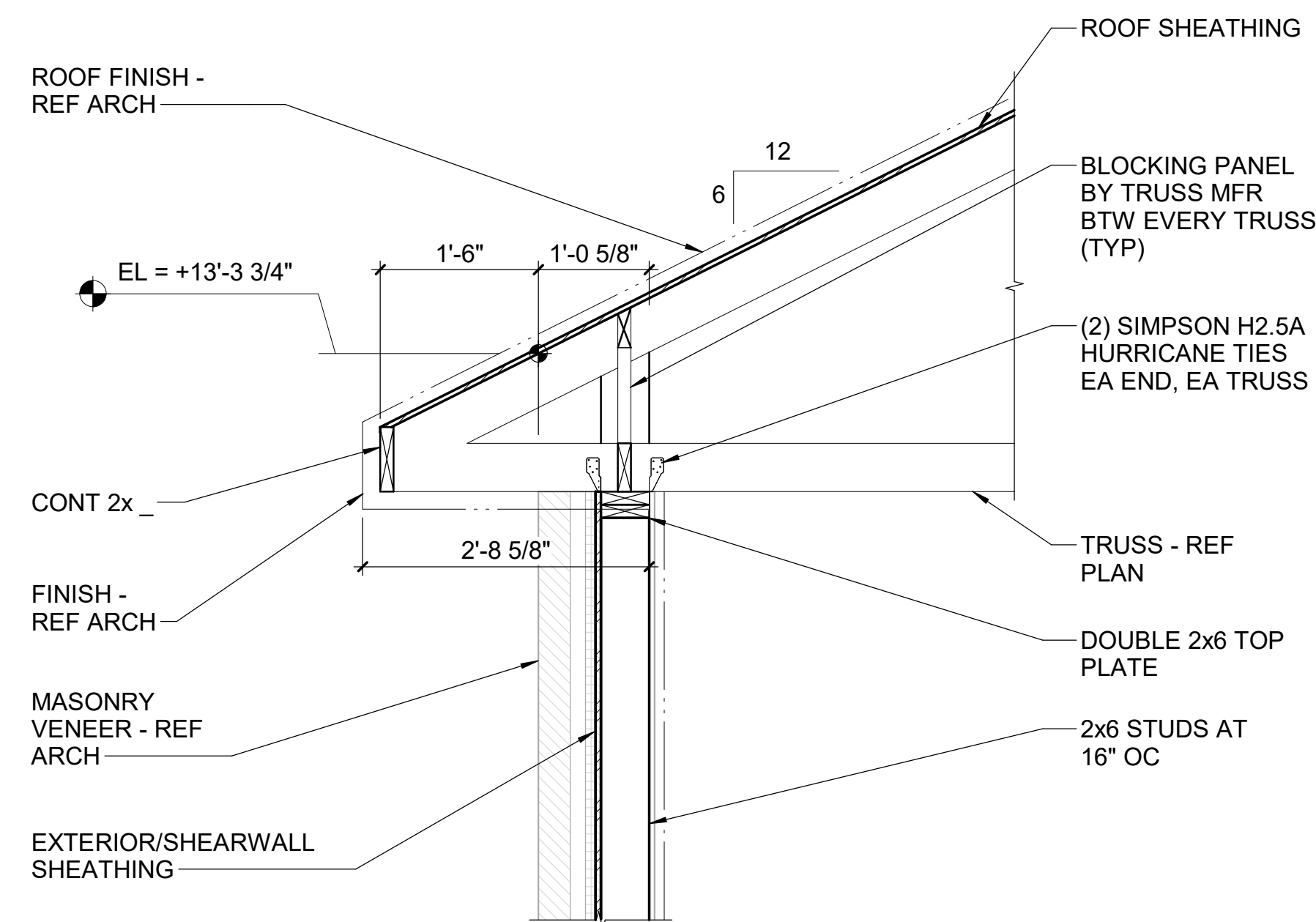
1 SECTION

3/4" = 1'-0"



2 SECTION

3/4" = 1'-0"



3 SECTION

3/4" = 1'-0"



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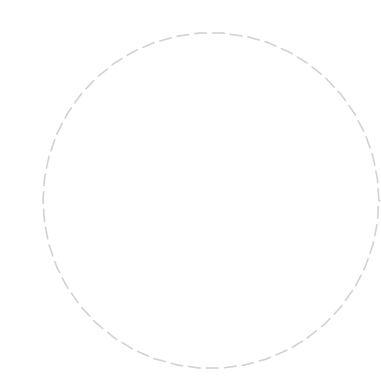
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UNCW Walton Drive
Student Village Recreation
Facilities Phase 4
1156 Walton Drive Wilmington, NC 28403
LS3P PROJECT: 7202-217800



REVISIONS:
Δ DATE DESCRIPTION

PLAN INFORMATION
SCO NO. 21-23669-01B
PROJECT NO. UNW21001
FILENAME
CHECKED BY Checker
DRAWN BY Author
SCALE 3/4" = 1'-0"
DATE 06/17/2022

SHEET
SECTIONS

S301

THE LINE SHOWN ABOVE IS EXACTLY
ONE INCH LONG AT THIS SHEETS
ORIGINAL PAPER SIZE

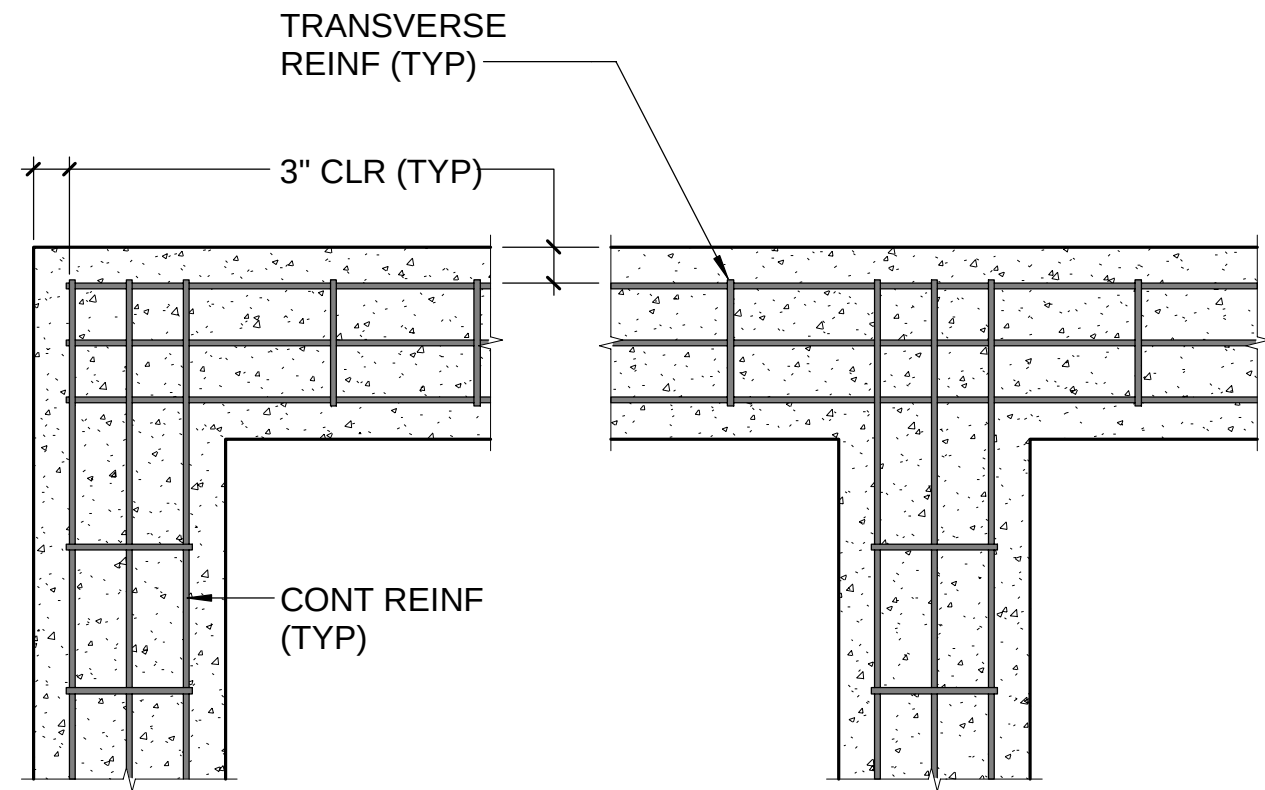
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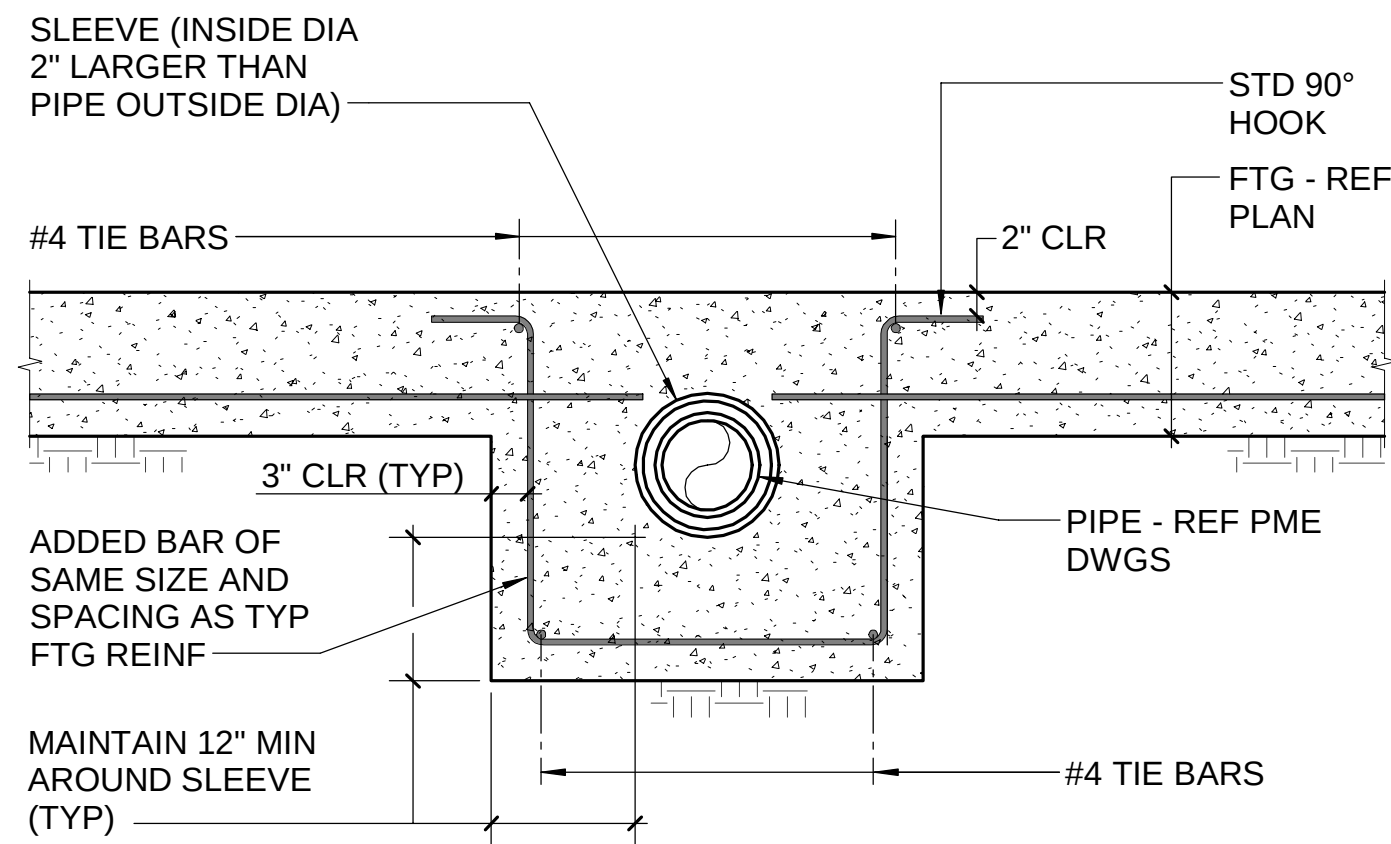
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TYPICAL CORNER AND INTERSECTION
DETAILS

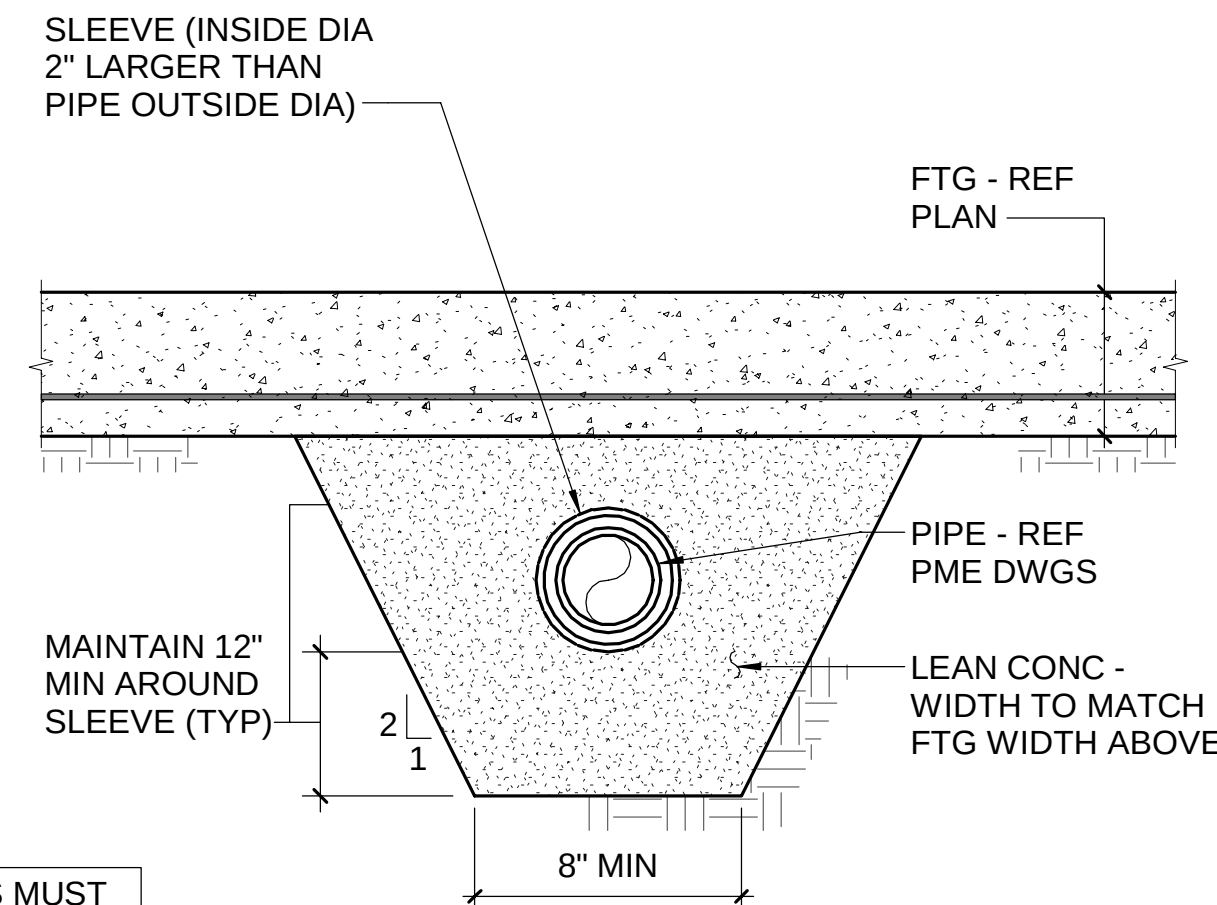
NTS



THROUGH FOOTING

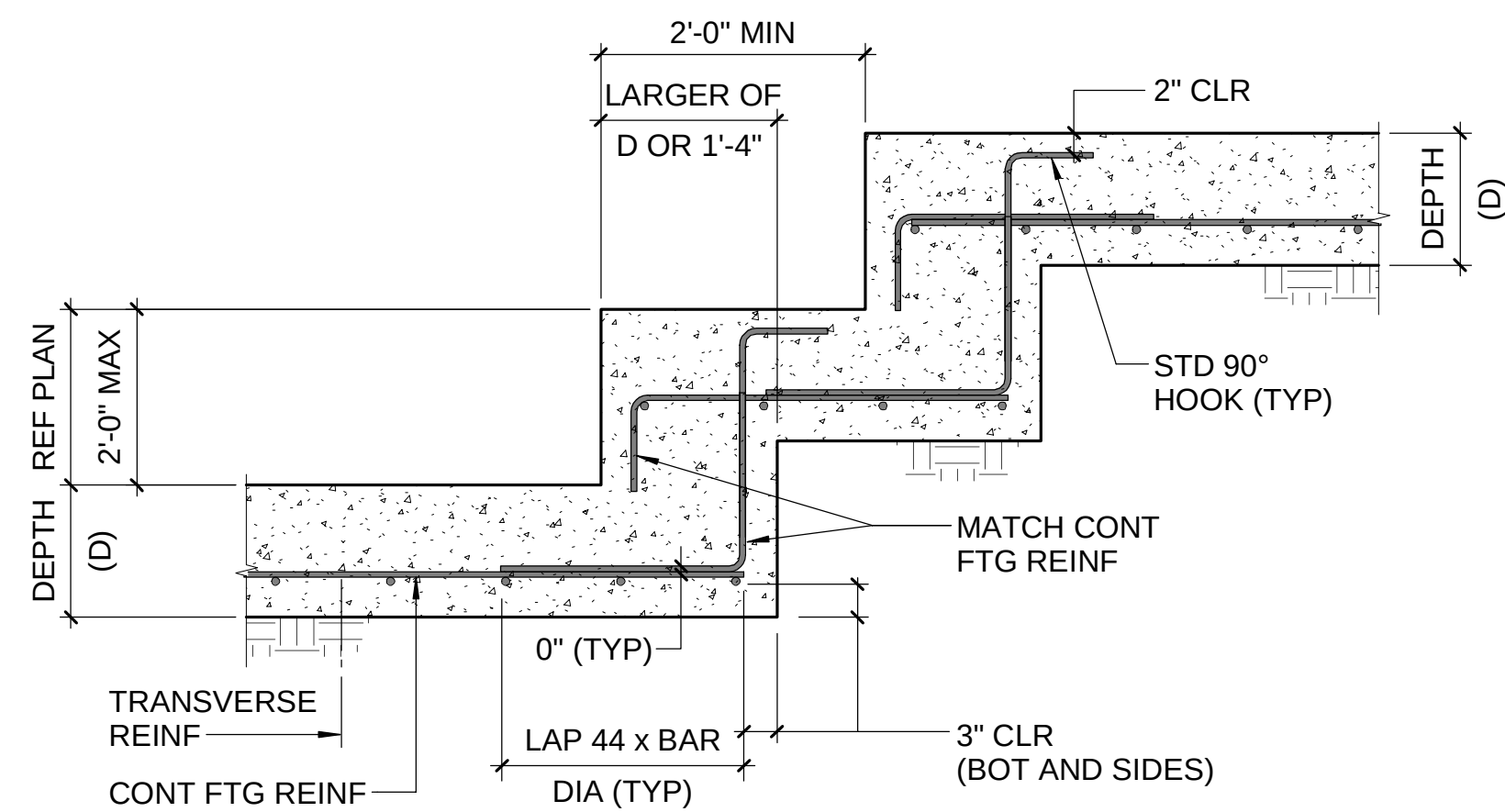
TYPICAL PIPE SLEEVE DETAILS

NTS



BELOW FOOTING

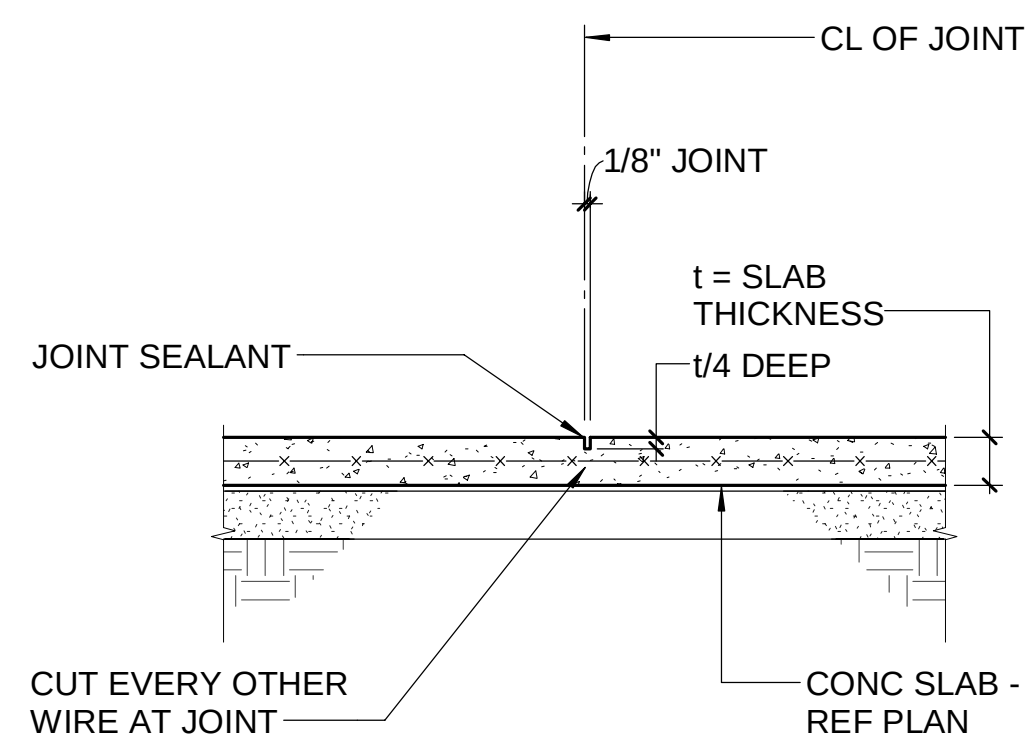
NOTE: SLEEVES MUST
NOT BE LOCATED IN OR
UNDER COL FTGS.



TYPICAL STEPPED FOOTING DETAIL

NTS

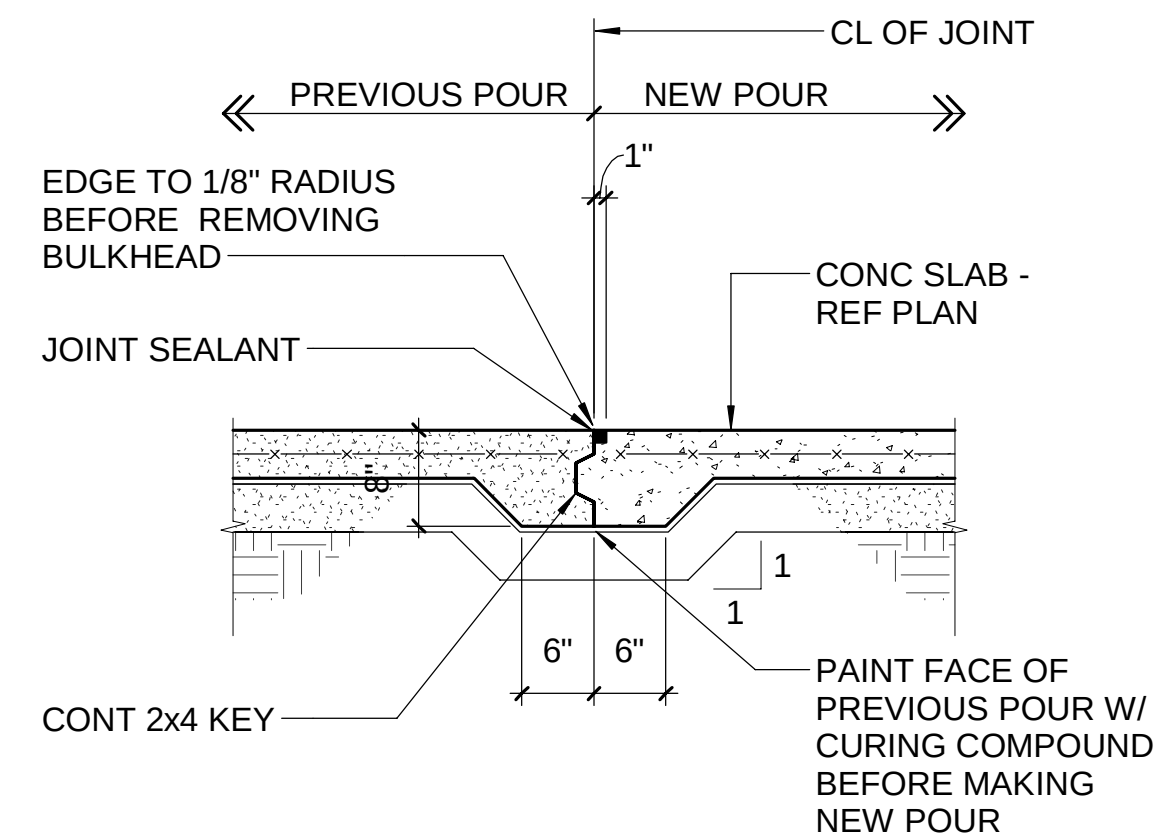
(DENOTED SF ON PLAN)



TYPICAL SAWED JOINT DETAIL

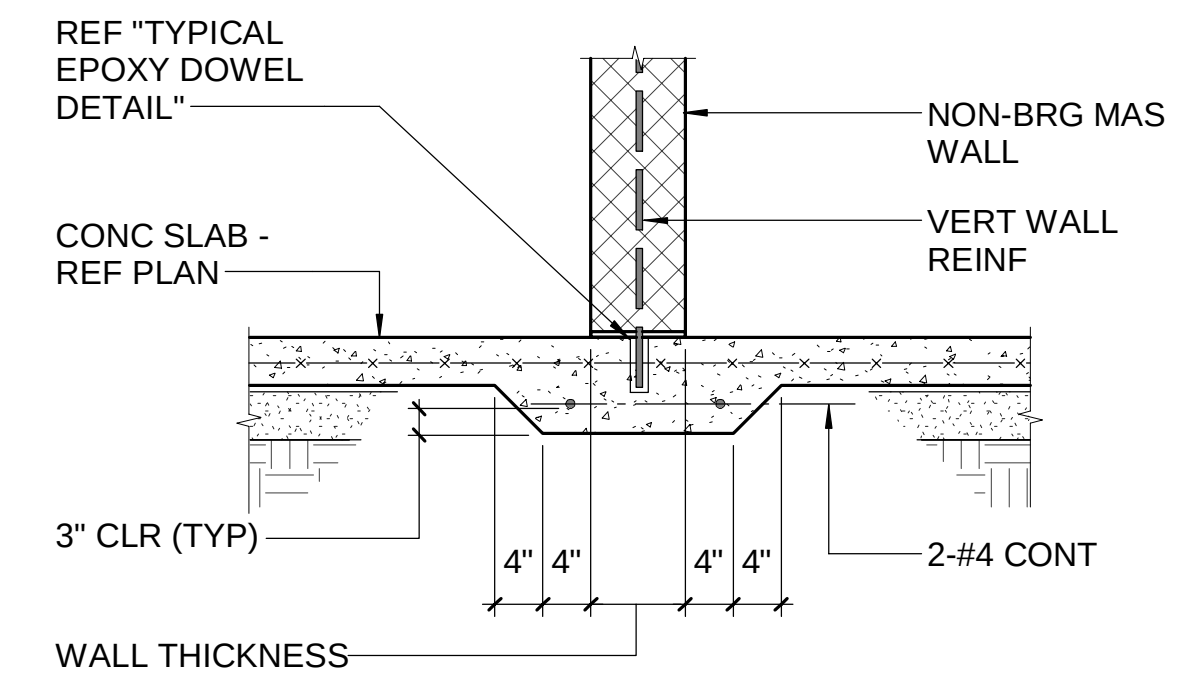
NTS

(DENOTED SJ ON PLAN)



TYPICAL KEYED CONSTRUCTION JOINT
DETAIL

(DENOTED KCJ ON PLAN)



TYPICAL THICKENED SLAB DETAIL

NTS

(DENOTED TS ON PLAN)



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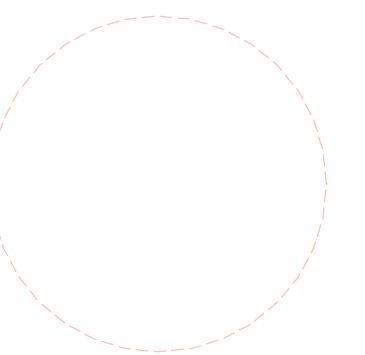
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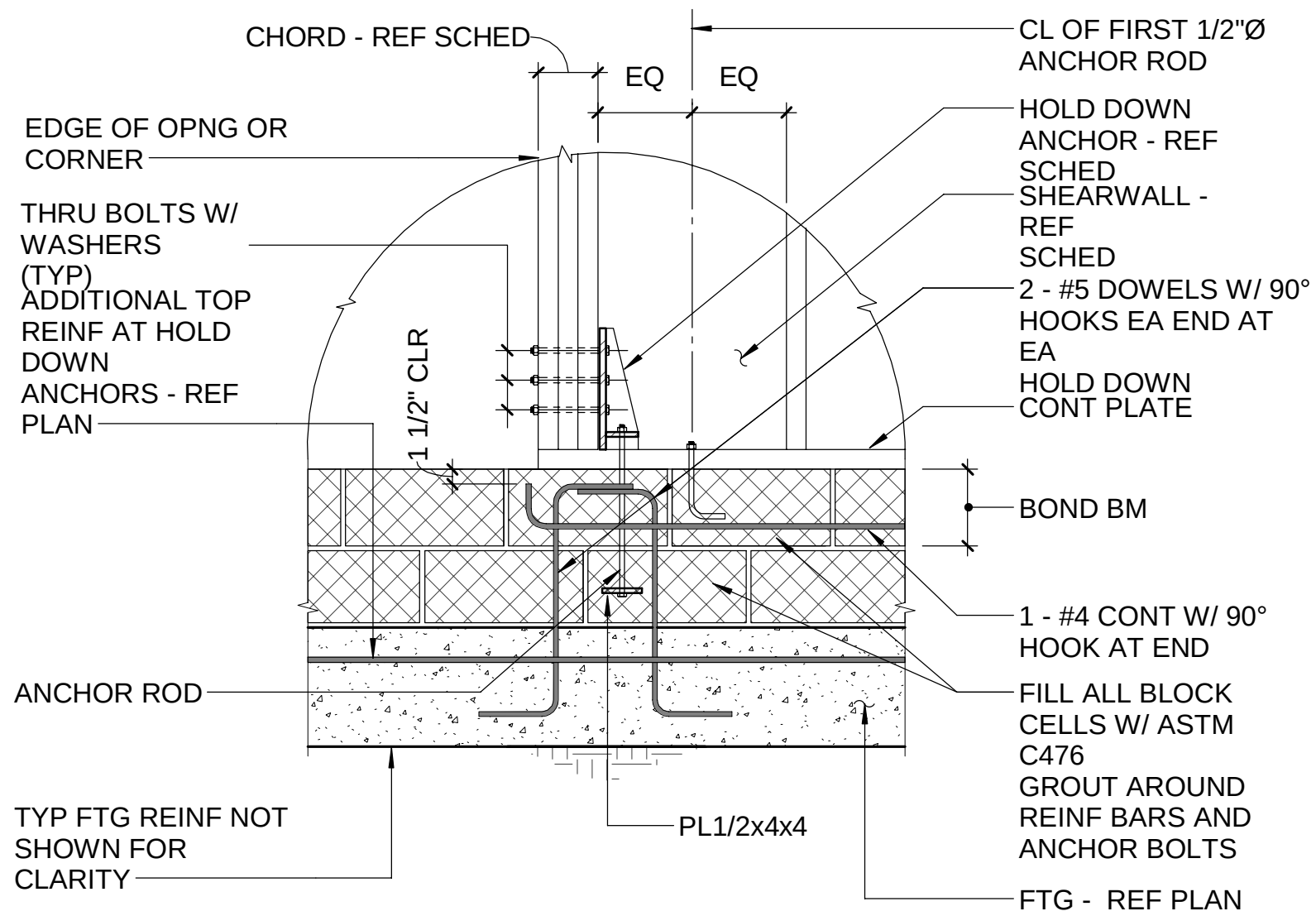
SHEET

TYPICAL DETAILS

S501

THE LINE SHOWN ABOVE IS EXACTLY ONE INCH LONG AT THIS SHEETS ORIGINAL PAPER SIZE

D



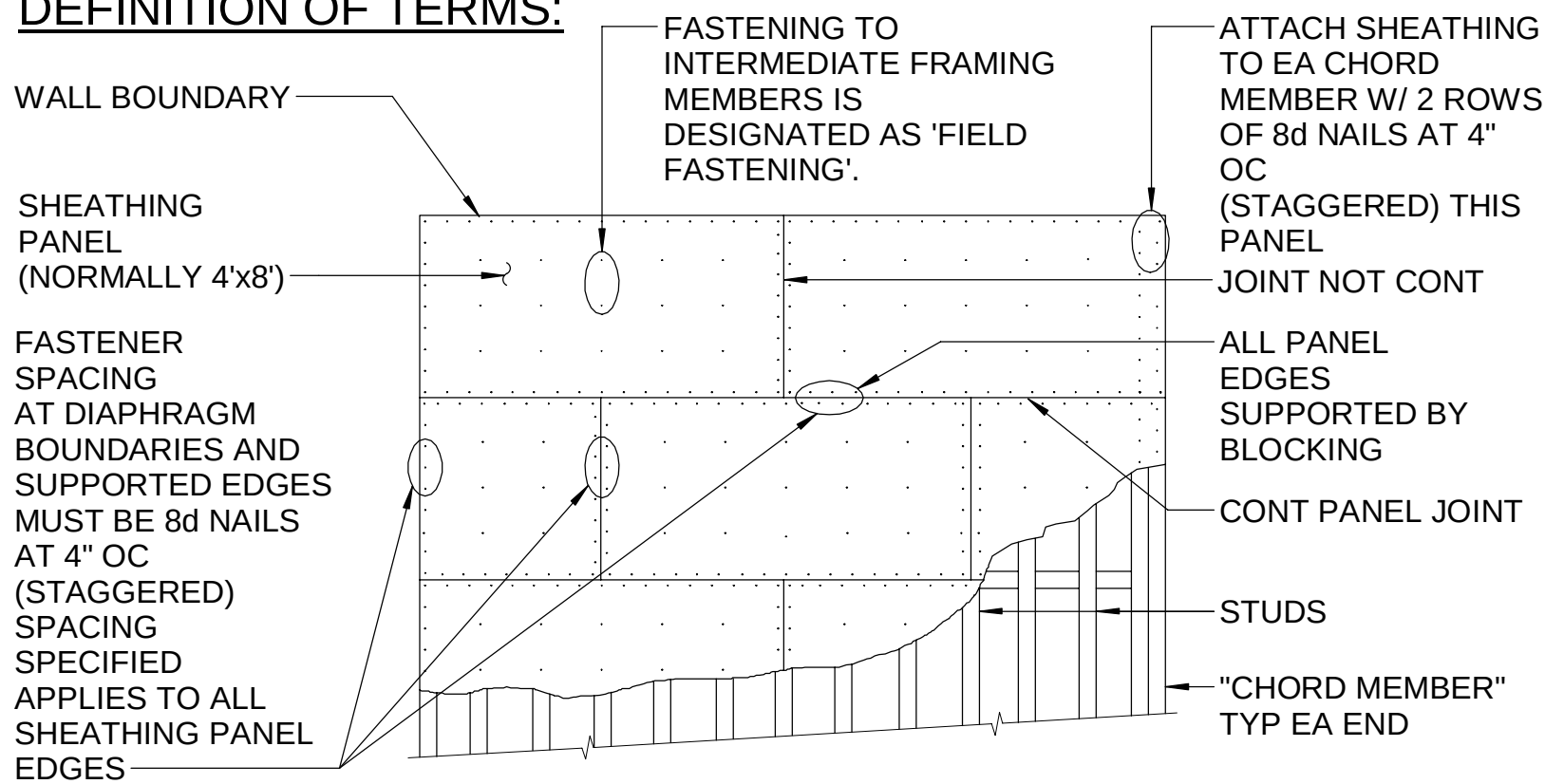
TYPICAL HOLD DOWN ANCHOR DETAIL

NTS

WALL SHEATHING NOTES:

- REFERENCE FOUNDATION AND FRAMING PLANS FOR HOLD DOWN ANCHOR LOCATIONS DENOTED THUS (⚡). REF "TYPICAL HOLD-DOWN ANCHOR DETAILS".
- ALL WALLS MUST HAVE PANEL EDGES BLOCKED WITH 4x4 POSTS. REF "TYPICAL WALL SHEATHING DIAPHRAGM DETAIL" BELOW.
- ALL WALLS SHALL HAVE HORIZONTAL BLOCKING AT MID-HEIGHT WITH 2x MEMBERS.

DEFINITION OF TERMS:



TYPICAL WALL SHEATHING DIAPHRAGM DETAIL

NTS

B

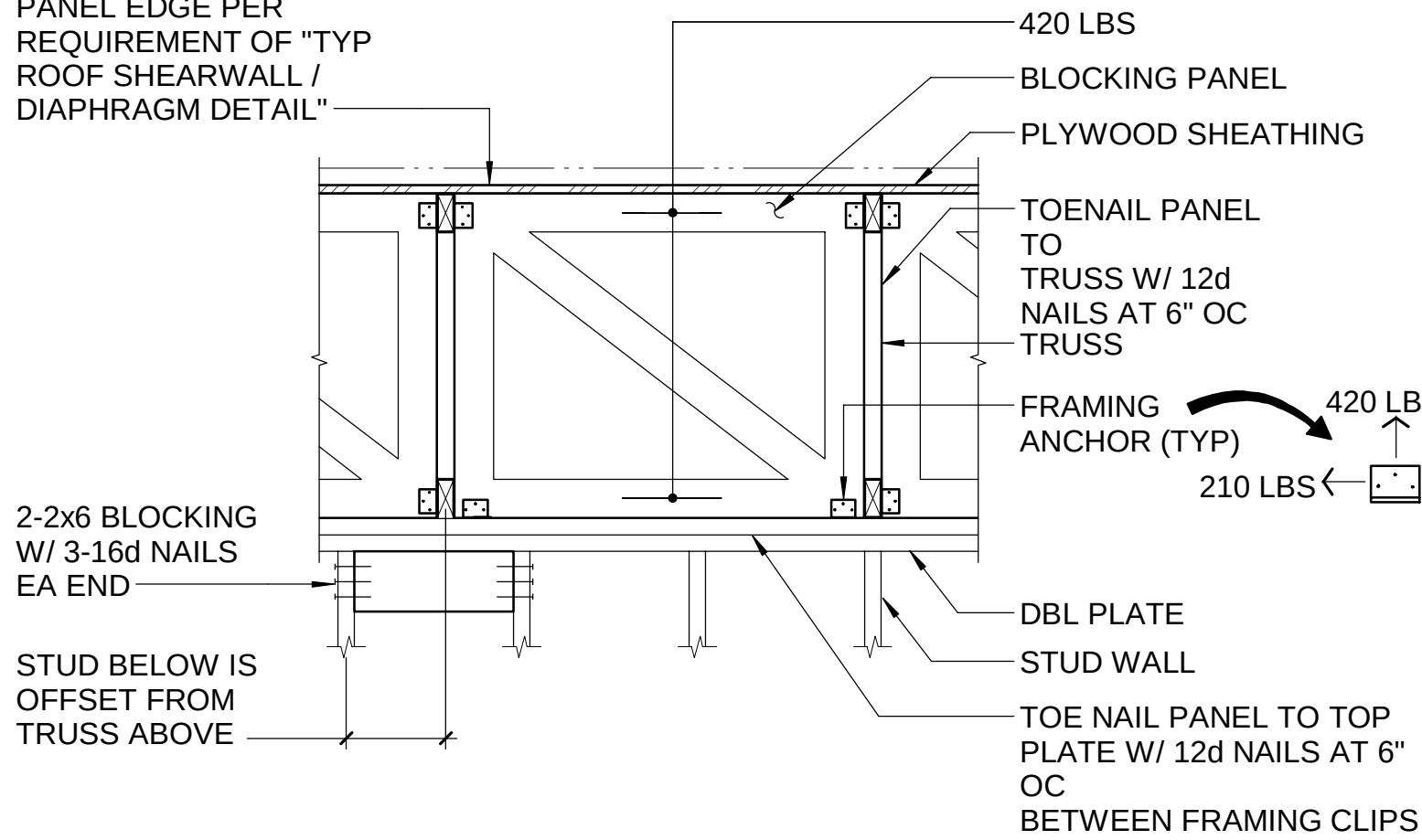
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SHEARWALL SCHEDULE

MARK	STUD SIZE	STUD SPACING	SHEAR PANEL EDGE FASTENERS	CHORD MEMBER EACH SIDE OF WALL	SIMPSON TYPE HOLDDOWN	ANCHOR BOLT	HOLDDOWN TO CHORD FASTENERS	SILL PLATE ANCHORAGE	SHEATHING
						DIAMETER			
SW-1	2x6	16" OC	8d AT 4" OC	(2) 2x6	HDU8-SDS2.5	7/8"	(20) 1/4x2 1/2" SDS	5/8"Ø LONG SCREW ANCHORS AT 24" OC	15/32" WOOD STRUCTRAL PANEL SHEATHING
SW-2	2x6	16" OC	8d AT 6" OC	2x6	HDU2-SDS2.5	5/8"	(6) 1/4x2 1/2" SDS	5/8"Ø LONG SCREW ANCHORS AT 24" OC	15/32" WOOD STRUCTRAL PANEL SHEATHING

NAIL ROOF SHEATHING TO BLOCKING PANEL AS A PANEL EDGE PER REQUIREMENT OF "TYP ROOF SHEARWALL / DIAPHRAGM DETAIL"



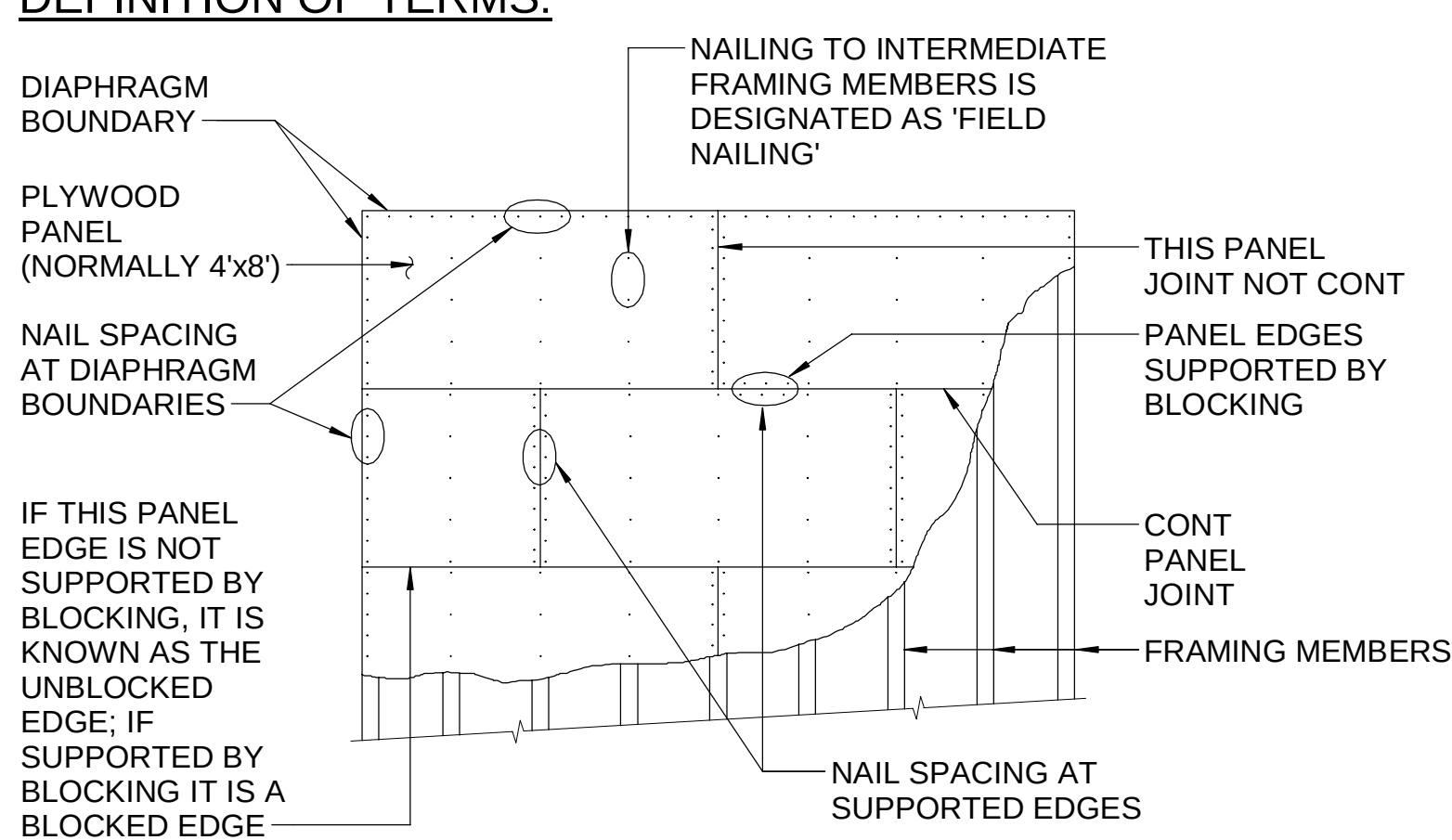
TYPICAL BLOCKING PANEL DETAIL

NTS

GENERAL: THE LATERAL LOADS ON THIS BUILDING ARE RESISTED BY A COMBINATION OF 'DIAPHRAGM' ACTION OF THE ROOF TO TRANSMIT LOADS IN THE HORIZONTAL PLANES, AND CMU SHEARWALLS TO TRANSMIT LOADS IN THE VERTICAL PLANES.

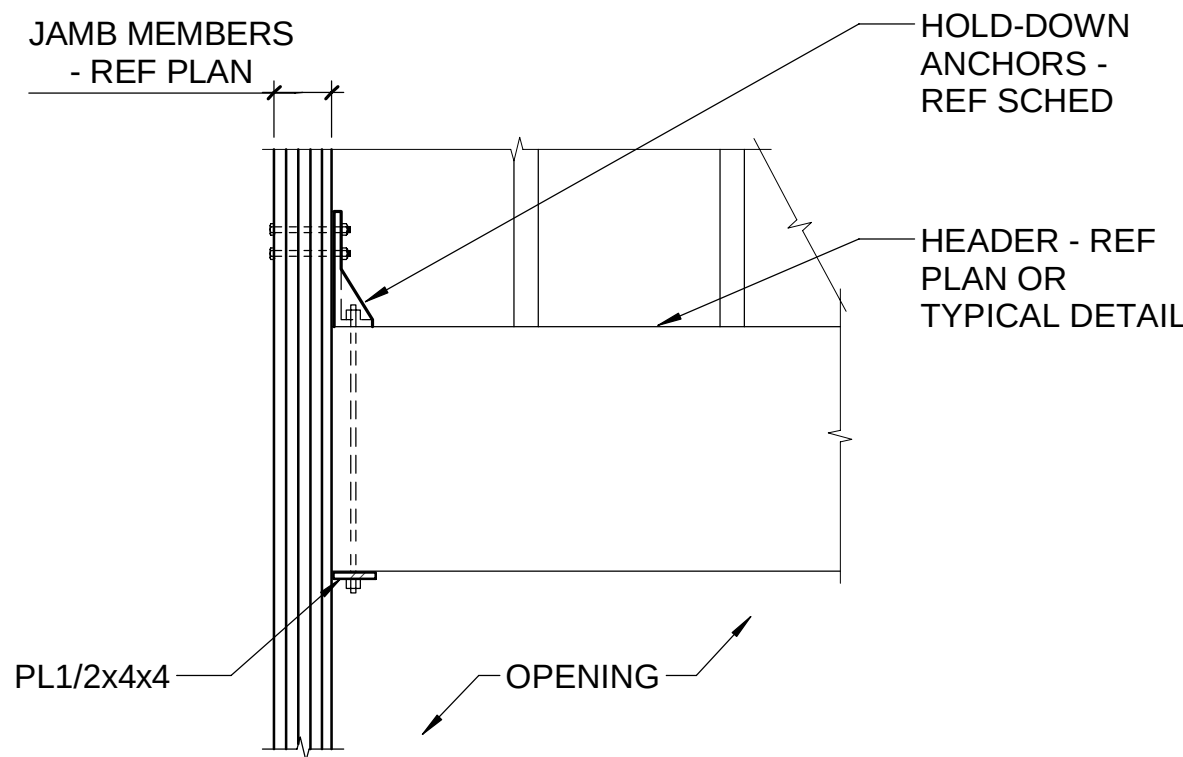
ROOF DIAPHRAGM NAILING SCHEDULE: EXCEPT WHERE OTHERWISE NOTED OR DETAILED, PLYWOOD ROOF SHEATHING SHALL BE NAILED W/ 6d NAILS AT 6" OC AT PANEL EDGES, 12" OC FIELD. BLOCK UNSUPPORTED EDGES WHERE INDICATED ON PLAN.

DEFINITION OF TERMS:



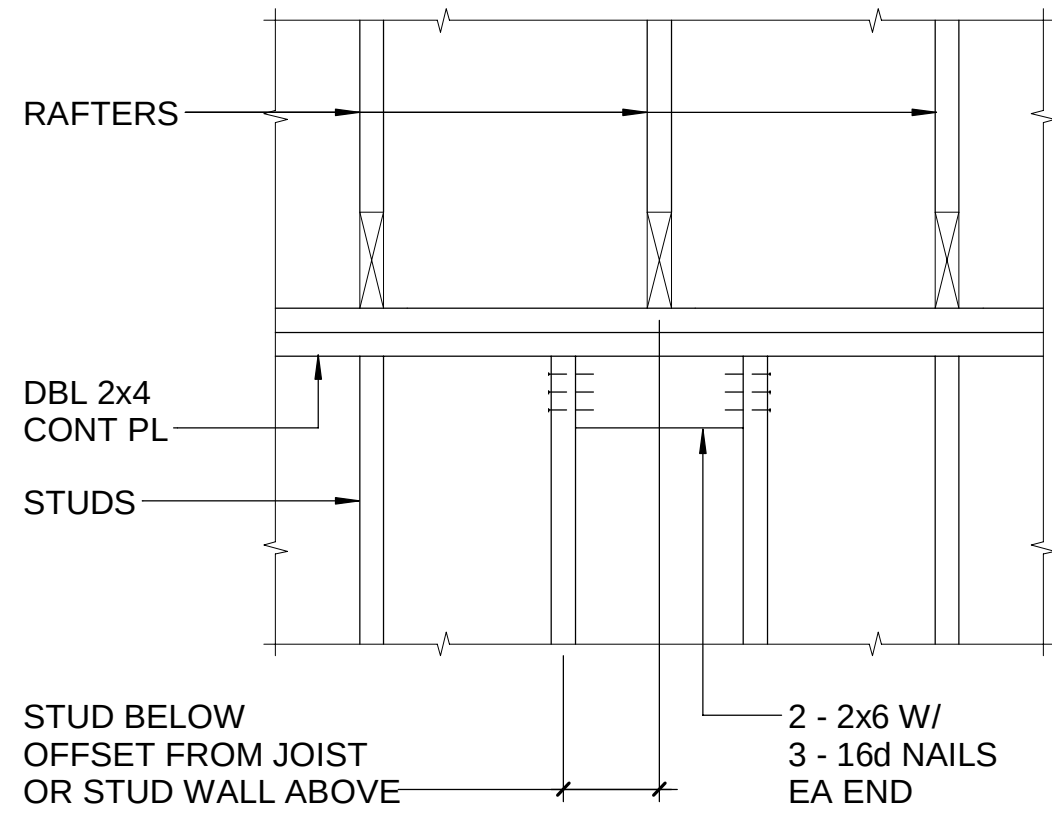
TYPICAL ROOF DIAPHRAGM DETAIL

NTS



TYPICAL HOLD DOWN ANCHOR DETAIL AT OPENINGS

NTS



TYPICAL OFFSET RAFTER BEARING OVER STUDS DETAIL

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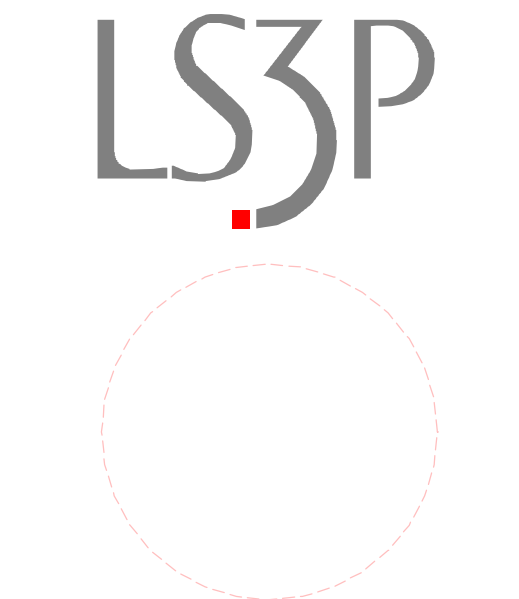
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TYPICAL DETAILS